PIERS IS EIGHT TIMES PIER DIAMETER WITHOUT AN ACCOMPANYING ENGINEERING LETTER

6. Site application design professional and dsa reviewer shall verify the structure to be located at least 20 FT FROM ANY HIGHER ADJACENT STRUCTURE IF GROUND SNOW LOAD IS GREATER THAN ZERO.

7. DESIGN COMPLIES WITH THE CONDITIONS OF EXCEPTION 1 OF ASCE 7-16 SECTION 11.4.8, ITEM 2.

8. APPROVED FIRE APPARATUS ACCESS ROADS SHALL EXTEND TO WITHIN 150 FEET OF ALL PORTIONS OF THE PERIMETER OF THE STRUCTURE PER CFC 503.1.1.

ARCHITECTURAL REQUIREMENTS:

DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	II B
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY POLIGON

RELATED BUILDING CODES AND STANDARDS:

TITLE 24 CODES:

2022 California Administrative Code (CAC).... 2022 California Building Code (CBC), Volumes 1 and 2 (Part 2, Title 24, CCR) (2021 International Building Code with 2022 California amendments) 2022 California Electrical Code. ..(Part 3, Title 24, CCR) (2020 National Electrical Code with 2022 California amendments) 2022 California Mechanical Code (CMC) ...(Part 4, Title 24, CCR) (2021 Uniform Mechanical Code with 2022 California amendments) 2022 California Plumbing Code (CPC). ...(Part 5, Title 24, CCR) (2021 Uniform Plumbing Code with 2022 California amendments) 2022 California Energy Code .(Part 6, Title 24, CCR) 2022 California Fire Code (CFC) ..(Part 9, Title 24, CCR) (2021 International Fire Code with 2022 California Amendments) 2022 California Existing Building Code (CEBC). ..(Part 10, Title 24, CCR) (Part 11, Title 24, CCR)

2022 California Green Building Standards Code 2022 California Referenced Standards Code ...

Title 19 CCR, Public Safety, State Fire Marshal Regulations

NFPA 72 - 2022 REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

2022 CBC, CHAPTER 35 **2022 CFC**, CHAPTER **80**

SCOPE OF WORK NARRATIVE:

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS

(Part 12, Title 24, CCR)

INSTRUCTIONS FOR ARCHITECTS PLANNING TO SUBMIT THESE PRE-CHECKED DRAWINGS TO DSA:

	STEP 1PROJECT INFORMATION
PROJECT NAME	
SCHOOL DISTRICT	
USE AND OCCUPANCY CLASSIFICATION	(PROPOSED OCCUPANCY: A1, A2, A3, A4, A5, E
OCCUPANT LOAD FACTOR	(15 SQFT/PERSON MAX; 5 SQFT/PERSON MIN FOR ANY A OCCUPAN 20 SQFT/PERSON MAX FOR B OR E OCCUPAN
TOTAL ROOF AREA	(MAXIMUM 4500 SQFT FOR ANY A OCCUPANCY, 10,000 SQFO FOR B OCCUPANCY, AND 5000 SQFT FOR E OCCUPAN
NUMBER OF OCCUPANTS	(MAXIMUM 300 FOR ANY A OCCUPANCY, 500 F B OCCUPANCY, AND 250 FOR E OCCUPAN

STEP 2 DESIGN OPTION					
ROOF DECK	[] MULTI-RIB	DEFAULT , WEIGHT 1.8 PSF			
	[] STANDING SEAM (SS)	WEIGHT 1.8 PSF			
GUTTERS	[] NO	DEFAULT			
	[] YES	SEE REK7.0 FOR DETAILS			
ELECTRICAL ACCESS	[] NO	DEFAULT			
	[] YES	SEE REK7.1 FOR DETAILS			
CLEAR HEIGHT	[] 8'	DEFAULT			
	[]OTHER	10' MAX			

STEP 3 SEISMIC ACCELERATION

93	(9)
S1	(g)
STEP 4 SEISMIC REGIONS	

1.406 < Ss <= 2.063	S1 <= 1.070	1.070 [] GREEN			
	<u>STEP</u>	5 TOTAL ROOF DEAD LOAD			
ROOF DECK		PSF	SEE STEP 2' 'ROOF DECK FO WEIGHT		

] WHITE

COLLATERAL	PSF	LIGHTING , FIRE SUPPRESSION, ETC
TOTAL	PSF	ADD 'ROOF DECK' AND 'COLLATERAL'
STE	P 6 LOAD SCENARIO	

TOTAL ROOF DEAD LOAD < 2.0 PSF	[] LOAD SCENARIO 2

TOTAL ROOF DEAD LOAD <= 3.5 PSF

STEP 7 PC STRUCTURE	
ROOF WIDTH <= 20	[] REK 20
20 < ROOF WIDTH <= 30	[] REK 30

STEP 8 STRUCTURE SIZE							
REK 20 REK 30							
[] 20'	DEFAULT	[] 30'		DEFAULT			
[]	OTHER 10' MIN; 20' MAX	[]	OTHER	20'-6" MIN; 30' MAX			
[]44'	2 BAYS	[]44'		2 BAYS			
[] 64'	3 BAYS	[] 64'		3 BAYS			
[]84'	4 BAYS	[]84'		4 BAYS			
	[] 20' [] [] 44' [] 64'	REK 20 [] 20' DEFAULT []OTHER 10' MIN; 20' MAX [] 44' 2 BAYS [] 64' 3 BAYS	REK 20 [] 20' DEFAULT [] 30' []OTHER 10' MIN; 20' MAX [] [] 44' 2 BAYS [] 44' [] 64' 3 BAYS [] 64'	REK 20 RE [] 20' DEFAULT [] 30' []OTHER 10' MIN; 20' MAX []OTHER [] 44' 2 BAYS [] 44' [] 64' 3 BAYS [] 64'			

	[] 44'	2 BAYS	[] 44'	2 BAYS		
ROOF LENGTH	[] 64'	3 BAYS	[] 64'	3 BAYS		
	[]84'	4 BAYS	[]84'	4 BAYS		
	[]OTHE	R	[]OTHER			
STEP 9 FOUNDATION TYPE						
	REK 20 REK 30					

FOUNATION TYPE

STEP 10 FOUNDATION SUMMARY						
	REK 20		REK 30			
[] LOAD SCENARIO 1	SPREAD PAD	[] LOAD SCENARIO 1 DRILLED PIER	[]LOAD SCENARIO 1 SPREAD PAD	[] LOAD SCENARIO 1 DRILLED PIER		
[] LOAD SCENARIO 2 PAD	SPREAD	[] LOAD SCENARIO 2 DRILLED PIER	[] LOAD SCENARIO 2 SPREAD PAD	[] LOAD SCENARIO 2 DRILLED PIER		

1 DRILLED PIER

[] SPREAD PAD

		<u>ST</u>	EP 11 SHI	EET INDEX				
BASE FRAME REK 20 SHEET INDEX REK 30 SHEET INDEX								
ROOF DECK	N	⁄IR		SS	N	1R	9	SS
FOUNDATION TYPE	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER
SELECT ONE	[]	[]	[]	[]	[]	[]	[]	[]
ORDER FORM	REK1.0	REK1.0	REK1.0	REK1.0	REK1.0	REK1.0	REK1.0	REK1.0
NOTES AND SPECIAL INSPECTIONS	REK1.1	REK1.1	REK1.1	REK1.1	REK1.1	REK1.1	REK1.1	REK1.1
FOUNDATION PLAN	REK2.0	REK2.1	REK2.0	REK2.1	REK2.2	REK2.3	REK2.2	REK2.3
FRAMING PLAN	REK3.0	REK3.0	REK3.0	REK3.0	REK3.1	REK3.1	REK3.1	REK3.1
FRAME CONNECTION DETAILS	REK4.0	REK4.0	REK4.0	REK4.0	REK4.1	REK4.1	REK4.1	REK4.1
ARCHITECTURAL VIEWS	REK5.0	REK5.0	REK5.0	REK5.0	REK5.1	REK5.1	REK5.1	REK5.1
ROOF CONNECTION DETAILS	REK6.0	REK6.0	REK6.1	REK6.1	REK6.0	REK6.0	REK6.1	REK6.1
MISC DESIGN OPTIONS	REK7.0	REK7.0	REK7.0	REK7.0	REK7.0	REK7.0	REK7.0	REK7.0
ELETRICAL CUTOUTS	REK7.1	REK7.1	REK7.1	REK7.1	REK7.1	REK7.1	REK7.1	REK7.1

	STEP 12 MULTIPLE STRUCTURES		
	ROOF WIDTH X LENGTH	QTY	+
MULTIPLE STRUCTURES			-

STEP 1: GENERAL PROJECT INFORMATION

- IDENTIFY PROJECT NAME AND SCHOOL DISTRIC IDENTIFY USE AND OCCUPANCY CLASSIFICATION

- THE USE AND OCCUPANCY DETERMINE THE MAXIMUM SQUARE FOOTAGE OF THE STRUCTURE - THE MAXIMUM SQUARE FOOTAGE IS ALSO LIMITED BY THE NUMBER OF OCCUPANTS IDENTIFY THE OCCUPANT LOAD PER TABLE 1004.5 IN THE CBC - IDENTIFY TOTAL ROOF AREA WHICH SHALL NOT EXCEED ALLOWABLE AREA PER TABLE 506.2 IN THE CBC.

- IDENTIFY EXPECTED NUMBER OF OCCUPANTS BASED ON THE ESTIMATED OCCUPANT LOAD
- TOTAL ROOF AREA DIVIDED BY OCCUPANT LOAD CAN DETERMINE NUMBER OF OCCUPANTS

STEP 2: DESIGN OPTIONS

SELECT ROOF DECK FOR YOUR PROJECT.

- "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK - "SS" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK

· SELECT WHETHER GUTTERS AND DOWNSPOUTS FROM POLIGON IS NEEDED FOR YOUR PROJECT - IF "YES", THEN INCLUDE SHEET REK7.0 IN THE DRAWING SET -SELECT WHETHER ELECTRICAL CUTOUTS ARE NEEDED FOR YOUR PROJECT

- SHEET REK7.0 SHOWS ELECTRICAL CUTOUT SIZE AND LOCATION CUTOUTS IN COLUMNS - SHEET REK7.1 HAS INSTRUCTIONS AND SHEET TO IDENTIFY WHICH COLUMNS - SHEET REK7.1 MUST BE FILLED OUT IN THE SUBMITTAL SET APPROVED BY DSA

IF NOTHING IS FILLED IN ON REK7.1, POLIGON WILL ASSUME CUTOTUS ARE ONLY IN COLUMN A1 (SEE 'FRAMING PLAN' FOR REFERENCE)

SELECT CLEAR HEIGHT (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE) - IF NOTHING IS SELECTED, POLIGON WILL ASSUME THE DEFAULT FOR EACH DESIGN OPTION

STEP 3: IDENTIFY THE Ss & S1 ACCELERATION (g) FOR YOUR PROJECT AND GEOTECHNICAL INFORMATION

- Ss & S1 VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES

- Ss & S1 VALUE DEPENDS ON PROJECT'S GEOGRAPHICAL LOCATION
- FIND Ss & S1 VALUES FOR YOUR PROJECT IN THE SITE SPECIFIC GEOTECHNICAL REPORT - FIND Ss & S1 VALUES FOR YOUR PROJECT USING https://asce7hazardtool.online/
- THIS PC IS NOT APPROVED FOR Ss VALUES GREATER THAN 2.063 (CONTACT POLIGON FOR ADDITIONAL OPTIONS)

STEP 4: IDENTIFY THE SEISMIC REGION FOR YOUR PROJECT

. The regions are dependant on the Ss & S1 Values determined in Step \Im - THE SEISMIC REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED (SEE TABLE TO THE LEFT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT

THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED · THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME TOTAL ROOF DEAD LOAD MUST BE LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 - CUT SHEETS OF ANY BOARDS, BOXES AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE, INCLUDING WEIGHTS AND DIMENSIONS ARE REQUIRED

STEP 6: IDENTIFY THE LOAD SCENARIO

3.5 PSF MAX DEAD LOAD

[] DRILLED PIER

[] LOAD SCENARIO 1

[] SPREAD PAD

- REFERENCE THE STEP 4 COLOR AND SELECT THE APPLICABLE LOAD SCENARIO · LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST, BUT DO AFFECT FOUNDATION SIZE

STEP 7: IDENTIFY PC STRUCTURE

- ROOF WIDTHS UP TO 20' WIDE USE THE "REK 20"

- ROOF WIDTHS UP TO 30' WIDE USE THE "REK 30" THE 20' AND 30' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL - MAXIMUM WIDTH IS 30'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)

STEP 8: IDENTIFY SITE SPECIFIC ROOF WIDTH AND LENGTH

- DO NOT EXCEED THE TOTAL ROOF AREA FROM STEP 1 (ROOF WIDTH MULTIPLIED BY ROOF LENGTH)

STEP 9: FOUNDATION TYPE

SELECT A FOUNDATION BASED THE DESIRED FOUNDATION TYPE SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION PRIOR TO APPROVAL FOUNDATION TYPE IMPACTS CONSTRUCTION (TIMING, SEQUENCE, COST, ETC.)

· FOUNDATION TYPE IMPACTS ANCHOR BOLT LENGTH (NOT PROVIDED BY POLIGON - REVIEW OF SITE-SPECIFIC SOILS REPORT TO EVALUATE APPLICABILITY OF FOUNDATION OPTIONS AVAILABLE

STEP 10: FOUNDATION SUMMARY

- USE THE SELECTIONS FROM STEP 6 AND STEP 9 TO SELECT THE APPROPRIATE FOUNDATION

STEP 11: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT

- IDENTIFY THE APPLICABLE SHEET INDEX

- INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS - EXCLUDE 'ELECTRICAL CUTOUTS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS

STEP 12: MULTIPLE STRUCTURES WITH THE SAME PC#

- FILL IN ROOF LENGTH AND WIDTH OF STRUCTURES AS WELL AS QUANTITY - UNO ON THE POLIGON DRAWINGS, POLIGON WILL ASSUME ALL DESIGN CRITERIA FOR EACH STRUCTURE IS THE SAME - CONTACT POLIGON FOR FURTHER INFORMATION

1 REK1.0 ORDER FORM 1 REK5.0 ARCHITECTURAL VIEWS - REK 20 2 REK1.1 NOTES AND SPECIAL INSPECTIONS ARCHITECTURAL VIEWS - REK 30 FOUNDATION PLAN SPREAD PAD - REK 20 13 REK6.0 ROOF CONNECTION DETAILS FOUNDATION PLAN DRILLED PIER - REK 20 14 REK6.1 ROOF CONNECTION DETAILS 5 REK2.2 FOUNDATION PLAN SPREAD PAD - REK 30 15 REK7.0 MISC DESIGN OPTIONS 6 REK2.3 FOUNDATION PLAN DRILLED PIER - REK 30 16 REK7.1 | ELECTRICAL CUTOUTS 7 REK3.0 FRAMING PLAN - REK 20 8 REK3.1 FRAMING PLAN - REK 30 9 REK4.0 FRAME CONNECTION DETAILS - REK 20 10 REK4.1 FRAME CONNECTION DETAILS - REK 30

MULTI-RIB ROOF PANEL (MCELROY)

ABBREVIATIONS:

AMERICAN CONCRETE INSTITUTE

OTAL SHEETS = 16

AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE	
ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER	F
AMERICAN SOCIETY FOR TESTING AND MAT'LS	ОС	ON CENTER] 3
AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.	
CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT	
COMPLETE JOINT PENETRATION	PD	POLIGON DRAWING	
CLEAR	PJ	PRETENSIONED JOINT]] .
DEGREE	PLCS	PLACES	
DIAMETER	PLT	PLATE	
DIMENSION	PSF	POUNDS PER SQUARE FOOT	
DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH	
EQUAL	QTY	QUANTITY	
FEET	REF	REFERENCE	
GAGE	SQ	SQUARE	
INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)	
KIPS PER SQUARE INCH	TYP	TYPICAL] 3
MAXIMUM	UNO	UNLESS NOTED OTHERWISE	
MINIMUM	USGS	U.S. GEOLOGICAL SURVEY] 3
MISCELLANEOUS	W/	WITH	
MILES PER HOUR			
	ASSEMBLY (INTERNAL REFERENCE) AMERICAN SOCIETY FOR TESTING AND MAT'LS AMERICAN WELDING SOCIETY CALIFORNIA BUILDING CODE COMPLETE JOINT PENETRATION CLEAR DEGREE DIAMETER DIMENSION DIVISION OF THE STATE ARCHITECT EQUAL FEET GAGE INCHES KIPS PER SQUARE INCH MAXIMUM MINIMUM MISCELLANEOUS	ASSEMBLY (INTERNAL REFERENCE) AMERICAN SOCIETY FOR TESTING AND MAT'LS OC AMERICAN WELDING SOCIETY OSHA CALIFORNIA BUILDING CODE COMPLETE JOINT PENETRATION PD CLEAR PJ DEGREE PLCS DIAMETER PLT DIMENSION PSF DIVISION OF THE STATE ARCHITECT EQUAL FEET GAGE INCHES KIPS PER SQUARE INCH MAXIMUM UNO MISCELLANEOUS W/	ASSEMBLY (INTERNAL REFERENCE) AMERICAN SOCIETY FOR TESTING AND MAT'LS AMERICAN WELDING SOCIETY OSHA CALIFORNIA BUILDING CODE PCF POUNDS PER CUBIC FOOT COMPLETE JOINT PENETRATION PD POLIGON DRAWING CLEAR PJ PRETENSIONED JOINT DEGREE PLCS PLACES DIAMETER DIMENSION PSF POUNDS PER SQUARE FOOT DIVISION OF THE STATE ARCHITECT PSI PACES GAGE SQ SQUARE INCHES SS STANDING SEAM ROOF PANEL (MCELROY) KIPS PER SQUARE INCH TYP TYPICAL MAXIMUM UNO UNLESS NOTED OTHERWISE MINIMUM USGS U.S. GEOLOGICAL SURVEY

SPECIFICATIONS

1.1 STRUCTURE DESCRIPTION

2. MEMBERS SIZES

PART 1 - GENERAL

1. GABLÉ ROOF (REK)

.2 DESIGN REQUIREMENTS

A. MEET THE DESIGN INTENT SHOWN ON THE PC DRAWINGS APPROVED FOR THIS PROJECT.

3. HIDDEN BOLTED CONNECTIONS BETWEEN STRUCTURAL MEMBERS 4. COLUMN ANCHORAGE SHALL INCLUDE FOUR (4) BOLTS IN COMPLIANCE WITH OSHA 1926.755(A)(1).

5. NO FIELD WELDING PERMITTED 6. NO FIELD PAINTING PERMITTED

A. STRUCTURE(S) BASED ON THE FOLLOWING PC DESIGN(S):

7. ROOF DIMENSIONS AND SLOPES 8. EXPOSED STEEL ROOF FASTENERS (IF APPLICABLE) POWDER COATED BY MANUFACTURER 9. ROOF DECK SPANS FROM PEAK TO EAVE AND PERMITS PROPER DRAINAGE WITHOUT DEBRIS BUILD-

1.3 SUBMITTALS

A. DRAWINGS AND CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE APPROPRIATE

B. ONLY MANUFACTURERS THAT SUBMIT DRAWINGS AND CALCULATIONS PRIOR TO BID SHALL BE

C. MANUFACTURER MUST BE ABLE TO SUBMIT APPROPRIATE LABORATORY TESTS FOR THE FOLLOWING: 1. FRAME FINISH REQUIREMENTS LISTED IN PART 2 OF THIS SPECIFICATION. 2. CERTIFIED MILL TEST REPORTS FOR STRUCTURAL STEEL (DESCRIBING THE CHEMICAL AND PHYSICAL

A. MANUFACTURER MUST HAVE IN-HOUSE ENGINEERING DEPARTMENT AND A PROFESSIONAL ENGINEER LICENSED IN THE APPROPRIATE STATE TO ANSWER TECHNICAL QUESTIONS.

1.5 QUALITY ASSURANCE

1. FABRICATION PROCEDURES SHALL COMPLY WITH APPLICABLE CODES AND LOCAL REGULATIONS. 2. REQUIRED STRUCTURAL TESTS AND SPECIAL INSPECTIONS INCLUDED ON THE PROJECT DSA-103 FORM B. MANUFACTURER QUALIFICATIONS

1. MINIMUM (10) YEARS ENGINEERING AND FABRICATING PRE-ENGINEERED STRUCTURES 2. MANUFACTURER OWNED AND OPERATED POWDER COAT PAINT FINISH SYSTEM 3. ALL AWS CERTIFIED WELDERS 4. FULL-TIME PROFESSIONAL ENGINEER ON STAFF LICENSED IN THE APPROPRIATE STATE

5. FULL-TIME AWS CERTIFIED ASSOCIATE WELDING INSPECTOR ON STAFF 6. FULL-TIME QUALITY ASSURANCE MANAGER ON STAFF

7. FULL-TIME LEED AP ON STAFF C. MANUFACTURER CERTIFICATIONS 1. PCI 4000 CERTIFICATION THROUGH POWDER COATING INSTITUTE (PCI)

3. CERTIFIED MILL TEST REPORTS FOR STRUCTURAL BOLTS.

.6 MANUFACTURER WARRANTY A. STRUCTURE MUST HAVE (10) YEAR LIMITED WARRANTY ON STEEL FRAME MEMBERS.

2. AISC CERTIFIED FABRICATOR

B. STRUCTURE MUST HAVE (10) YEAR LIMITED WARRANTY ON PAINT SYSTEM. C. PASS THROUGH WARRANTY OF ROOFING MANUFACTURER SHALL BE PROVIDED UPON REQUEST.

PART 2 - PRODUCTS 2.1 MANUFACTURER

A. ACCEPTABLE MANUFACTURERS

1. POLIGON, A DIVISION OF PORTERCORP. A. 4240 N 136TH AVE., HOLLAND, MI 49424; 616-399-1963; WWW.POLIGON.COM.

I. FOR POLIGON STRUCTURES IN NORTHERN CALIFORNIA, THE LOCAL REPRESENTATIVE IS

ALL ABOUT PLAY (WWW.PLAYGROUNDPROS.COM). EMAIL AAP@PLAYGROUNDPROS II. FOR POLIGON STRUCTURES IN SOUTHERN CALIFORNIA, THE LOCAL REPRESENTATIVE IS

MIRACLE PLAYGROUND SALES (MIRACLEPLAYGROUNDSALES.COM EMAIL SALES@MIRACLEPLAYGROUND.COM OR CALL (951) 695-4515 1. THE ENGINEERING FOR THIS STRUCTURE IS ONLY APPLICABLE IF POLIGON SUPPLIES THE MATERIAL

2. IF THE CONTRACTOR ELECTS TO SUBSTITUTE A DIFFERENT STRUCTURE, THEY ARE RESPONSIBLE TO **OBTAIN THE NECESSARY DSA APPROVAL WITH:** A. NO COST TO THE DISTRICT OR ARCHITECT

B. NO CHANGE TO THE CONSTRUCTION SCHEDULE 3. SUBSTITUTIONS MUST BE APPROVED A MINIMUM OF (10) DAYS BEFORE BID.

4. ALL APPROVED MANUFACTURERS SHALL BE NOTIFIED IN WRITING BEFORE THE BID DATE. 5. SUBSTITUTE MANUFACTURERS SHALL NOT BE ALLOWED TO BID WITHOUT WRITTEN

NOTIFICATION. 6. SUBSTITUTE MANUFACTURERS MUST MEET "MANUFACTURER QUALIFICATIONS" LISTED IN

PART 1 OF THIS SPECIFICATION. 7. SUBSTITUTE MANUFACTURERS MUST PROVIDE PROOF OF "MANUFACTURER CERTIFICATIONS" ABOVE.

8. SUBSTITUTE MANUFACTURERS MUST PROVIDE PAINT FINISH DESCRIBED IN "FRAME FINISH"

2.2 FRAME

A. MATERIALS 1. ANCHOR BOLTS: SEE DRAWINGS FOR REQUIREMENTS. ANCHOR BOLTS NOT PROVIDED BY 2. STRUCTURAL STEEL: SEE DRAWINGS FOR REQUIREMENTS.

3. STRUCTURAL BOLTS: SEE DRAWINGS FOR REQUIREMENTS. 1. FRAME FINISH: POLI-5000 POWDER COAT. NO FIELD PAINTING PERMITTED.

A. COMPONENTS SHALL BE CLEANED, PRE-TREATED, AND FINISHED AT A FACILITY OWNED AND DIRECTLY SUPERVISED BY THE MANUFACTURER. B. COMPONENTS SHALL BE SHOT BLASTED TO SSPC-SP10 NEAR-WHITE BLAST CLEANING. SSPC-SP2

HAND TOOL CLEANING WILL NOT BE AN ACCEPTABLE ALTERNATIVE. COMPONENTS SHALL BE PRETREATED IN A (3) STAGE IRON PHOSPHATE OR EQUAL WASHER. D. COMPONENTS SHALL RECEIVE EPOXY PRIMER COAT FOR SUPERIOR CORROSION PROTECTION. COMPONENTS SHALL RECEIVE TOP COAT OF SUPER DURABLE TGIC POWDER COAT.

FINISH SHALL NOT HAVE ANY VOC EMISSIONS. G. MANUFACTURER SHALL BE ABLE TO PRODUCE DOCUMENTATION STATING SAMPLE PRODUCTION COMPONENTS HAVE BEEN TESTED TO MEET THE FOLLOWING: I. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654 TO 10,000 HOURS WITH NO CREEP

FROM SCRIBE LINE AND RATING OF 10. II. HUMIDITY RESISTANCE PER ASTM D2247-02 TO 5,000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING. III. COLOR/UV RESISTANCE PER ASTM G154-04 TO 2,000 HOURS EXPOSURE, ALTERNATE

CYCLES WITH RESULTS OF NO CHALKING, 75% COLOR RETENTION, COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2,000 HOURS EXPOSURE). 2. FRAME COLOR: DETERMINED BY DISTRICT.

C. FABRICATION 1. FABRICATE COMPONENTS TO PERMIT BOLTED CONNECTIONS ON SITE. NO FIELD WELDING 2. LABEL EACH MEMBER WITH UNIQUE PART NUMBER TO STREAMLINE ERECTION.

2.3 ROOF

1. ROOF MATERIAL: SEE DRAWINGS FOR REQUIREMENTS. 2. ROOF HARDWARE: SEE DRAWINGS FOR REQUIREMENTS

1. ROOF FINISH: KYNAR 500 HIGH-PERFORMANCE RESIN-BASED PAINT. 2. ROOF COLOR: DETERMINED BY OWNER.

3. WELDING REQUIREMENTS: SEE DRAWINGS FOR REQUIREMENTS

2.4 MISCELLANEOUS A. MATERIALS

I. CONCRETE MATERIAL: SEE DRAWINGS FOR REQUIREMENTS. CONCRETE NOT PROVIDED BY MANUFACTURER.

PART 3 - EXECUTION

3.1 STORAGE AND HANDLING A. PROTECT MATERIAL AFTER DELIVERY FROM WEATHER, SUNLIGHT, AND DAMAGE

B. ELEVATE MATERIAL TO ALLOW CIRCULATION AND REDUCE MOLD, FUNGI DECAY, AND INSECT INFESTATION. C. HANDLE MATERIAL WITH PROTECTIVE STRAPS OR PADDED FORKLIFT. HANDLING MATERIAL WITH CHAIN OR CABLE WILL NOT BE ACCEPTED AND MAY VOID MANUFACTURER'S WARRANTY. D. TO PREVENT MOISTURE DAMAGE TO ANY WOOD MATERIAL (IF APPLICABLE), KEEP WOOD PACKAGED BEFORE INSTALLATION AND COVER IMMEDIATELY WITH ANY SECONDARY ROOF MATERIAL.

COMPLY WITH ALL APPLICABLE OHSA REQUIREMENTS

A. INSTALL COMPONENTS ACCORDING TO MANUFACTURER'S INSTALLATION DRAWINGS AND THESE SPECIFICATIONS.

. ANCHOR BOLT AND COLUMN LAYOUT IS CRITICAL. C. COMPLY WITH SPECIFIC BOLTING INSTALLATION REQUIREMENTS PROVIDED ON DRAWINGS REQUIRING THE CONTRACTOR TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR

AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. D. NO FIELD SLOTTING OR OPENING OF HOLES WILL BE ALLOWED. TOLERANCES ON STEEL STRUCTURAL MEMBERS ARE SET ACCORDING TO AISC CONSTRUCTION PRACTICES, FOLLOWED DURING FABRICATION, AND CANNOT BE INCREASED.

E. AFTER INSTALLATION, RESTORE DAMAGED SURFACES TO THE ORIGINAL CONDITION USING TOUCH-UP PAINT PROVIDED BY MANUFACTURER. IF THE ARCHITECT DOES NOT ACCEPT THAT, REPLACE DAMAGED MATERIAL AT NO COST TO THE DISTRICT. COORDINATE AS REQUIRED WITH OTHER DISCIPLINES (ELECTRICAL, PLUMBING, ETC.)

A. DO NOT ATTEMPT ANY FIELD CHANGES TO THE STRUCTURE WITHOUT FIRST CONTACTING THE MANUFACTURER

3.4 QUALITY CONTROL A. TESTS AND INSPECTIONS DURING ERECTION ARE NOT REQUIRED BY THE MANUFACTURER, BUT MAY BE

REQUIRED BY OTHERS. B. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH DSA PR 13-01. DO NOT PROCEED UNTIL UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED

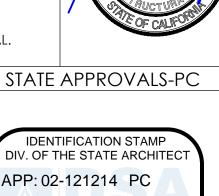


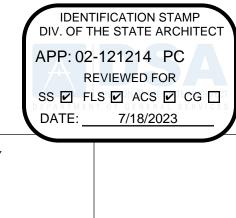
STATE APPROVALS-SITE











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GENERAL:

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE
- 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
- 10. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIRMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
- 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
- 14. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.
- 15. WHEN A SITE-SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE

STRUCTURAL AND MISCELLANEOUS STEEL:

- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360-16 AND 303-16 REFERENCED BY THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B UNLESS NOTED OTHERWISE.
- 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI.
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, PLATES AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.
- 6. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
- 7. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1
- 8. ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
- 9. ROOF DECK SHALL CONFORM TO ATSM A792, Fy = 50 KSI.

PROPER MATERIAL ID AND WELDING.

- 10. MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).
- 11. SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

WELDING:

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED.
- 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (O° F).
- 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE
- 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 (A325 TYPE 1) HIGH STRENGTH BOLTS (UNO) AND SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329.

- . HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLANCE WITH CBC 2213A.1.
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
- 4. ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
- 5. HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563 AND SHALL BE GALVANIZED PER ASTM F2329.
- 6. HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436 AND SHALL BE GALVANIZED PER ASTM F2329.
- THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
- A. PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
 - 1. TURN-OF-NUT PRETENSIONING
 - 2. CALIBRATED WRENCH PRENTENSIONING
 - DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
- B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED
- JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

FOUNDATIO

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER 2022 CBC TABLE 1806A. 2.
- 2. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557. FLOODING NOT PERMITTED.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECCESSARY TO SUPPORT CUT AND/OR FILL
- BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.

 4. STRUCTURES SHALL BE SETBACK FROM ADAJCENT SLOPES TO PROVIDE FIRM MATERIAL FOR EMBEDMENT AND FOR
 - PROTECTION FROM SLOPE DRAINAGE, EROSION, AND SHALLOW FAILURES.
 - A. BOTTOM OF ASCENDING SLOPE: THE SMALLER OF HALF THE HEIGHT OF THE SLOPE AND 15FT MEASURED FROM THE FACE OF THE STRUCTURE TO THE TOE OF THE SLOPE

 B. TOP OF DECENDING SLOPE: THE SMALLER OF A THIRD OF THE HEIGHT OF THE SLOPE AND 40 FT MEASURED FROM THE FACE OF THE FOOTING TO THE TOP OF THE SLOPE
 - ALTERNATE SETBACKS ARE PERMITTED, SUBJECT FOR APPROVAL. A GEOTECHNICAL INVESTIGATION MAY BE REQUIRED.
- 5. STRUCTURES PLACED ON LIQUIFIABLE SOILS OR SITE CLASS F MAY NOT BE SUBMITTED FOR AN OVER THE COUNTER

CONCRETE:

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

MINIMUM STRENGTH f'c (28 DAYS)	EXPOSURE CATEGORY	MAXIMUM W/C RATIO	SLUMP (± 1")	UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI	F3, S3, W2, C2	0.4	4"	150 PCF

- 2. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA
- 3. AGGREGATES SHALL CONFORM TO ASTM C33. MAX AGGREGATE SIZE = 1".
- 4. CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) WITH A MAXIMUM EXPANSION OF 0.040%, FOR SULFATE
- 5. ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE PROHIBITED.
- 6. CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318-19 SECTION 19.3.3.
- 7. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
- 8. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 9. CONCRETE SHALL BE PROPORTIONED PER ACI 318-19 26.4.
- 10. CONCRETE SHALL BE TESTED PER CBC 1910A.1, 1705A.3, AND ACI 318-19 26.13. BATCH PLANT INSPECTION NOT REQUIRED. CONTRACTOR SHALL IMPLEMENT WEIGHTMASTER AND BATCH TICKET REQUIREMENTS OF CBC 1705A.3.3.1.

REINFORCING STEEL:

- REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615, (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS:

 GR 60: (#4 BARS AND LARGER)
- 2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:

		_
	CAST AGAINST EARTH	
В.	CAST AGAINST FORM BELOW GRADE	2"
C.	FORMED SLABS (#11 BAR & SMALLER)	3/4"
	SLABS ON GRADE (FROM TOP OF SLAB)	
E.	COLUMNS AND BEAMS (MAIN BARS)	2"
F.	WALLS EXPOSED TO WEATHER (#6-#18 BARS)	2"
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- (#5 & SMALLER)....... 11/2"

 G. NOT EXPOSED TO WEATHER (#11 & SMALLER)....... 3/4"
- 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE
- 5. FOR #6 BARS AND SMALLER, REINFORCING SHALL BE LAP SPLICED 45 BAR DIA MINIMUM IN CONCRETE. FOR #7 BARS AND LARGER, REINFORCING SHALL BE LAP SPLICED 55 BAR DIAMETERS MINIMUM IN CONCRETE. ALL LAP SPLICES MUST COMPLY WITH A CL 318-19.
- 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN
- 7. WELDING OF REINFORCING IS NOT ALLOWED
- 8. REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1910A.2.

POWDER COATED AND EPOXY PRIMED FINISH:

- 1. ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
- 2. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).
- 3. PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
- PARIS PREIREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
 EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
- 5. TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR
- CUSTOM COLOR).

 6. SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA:
 - A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654

 1. 10000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10
 - B. HUMIDITY RESISTANCE PER ASTM D2247-02

 1. 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING
 - C. COLOR/UV RESISTANCE PER ASTM G154-04

 1. 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS EXPOSURE

CONSTRUCTION NOTES

- . A DSA-CERTIFIED CLASS 2 INSPECTOR IS REQUIRED FOR THIS PROJECT.
- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQURIED BY SECTION 4-338, PART 1, TITLE 24 CCR AND DSA IR A-6.
- 3. A "DSA-CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-
- 4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) SHALL CONDUCT ALL THE REQUIRED TEST AND INSPECTIONS FOR THE PROJECT.

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEER RESPONIBILITY

- FOR THE SITE-SPECIFIC PROJECT, NEITHER POLIGON OR GHD ARE THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE
- 2. FOR THE SITE-SPECIFIC PROJECT, GHD AND POLIGON'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC ONLY.
- 3. STRUCTURAL OBSERVATION OF CONSTRUCTION IPECIFICALLY EXCLUDED FROM GHD AND POLIGON'S RESPONSIBILITY FOR THE SITE-SPECIFIC PROJECT.
- 4. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING MAY BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATIONS OF CONSTRUCTION, REVIEW OF INSPECTIONS REPORTS, AND SIGNING OFF ON THE VERIFIED REPORT FOR COMPLETED WORK.
- 5. POLIGON WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

SPECIAL INSPECTION NOTES:

- THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
- 2. COSTS OF THE PROJECT INSPECTOR AND THE TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
- 3. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH DSA PR 13-01.
- 4. ON APPROVED PC DRAWINGS, THE STATEMENT OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS (FORM DSA-103) BELOW<u>IS ONLY AN EXAMPLE.</u>
 ON APPROVED PC DRAWINGS, THE EXAMPLE FORM DSA-103 MUST BE CROSSED OUT BEFORE THE PC DRAWINGS CAN BE APPROVED.

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d. Test concrete (f'c). e. Batch plan inspection: S/A1. STRUCTURAL STE Test or Special Inspectio a. Verify identification of all • Mill certificates indicate requirements. • Material sizes, types and b. Test unidentified materia c. Examine seam welds of d. Verify and document stee construction documents. S/A2. HIGH-STRENGTH E Test or Special Inspection a. Verify identification mand of compliance conform to an an of compliance conform to a compliance. S/A3. WELDING: Test or Special Inspection S/A3. WELDING: Test or Special Inspection a. Verify weld filler material markings per AWS design documents and the WPS. b. Verify weld filler material markings per AWS design documents and the WPS. b. Verify weld filler material markings per AWS design documents and the WPS. c. Verify WPS, welder question of the compliance. C. Verify WPS, welder question of the compliance of	Lure of the concrete. Lorarcriet (ft). In plan inspection: See Notes SI Default of 'Continuous' per 1705A.3.3. if approved by DSA batch plan inspection: See Notes SI Default of 'Continuous' per 1705A.3.3. if approved by DSA batch plan inspection may be reduced to Periodic subject to requirements in 1705A.3.3.1. or eliminated per 1705A.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.2. See IR 17-13. (See Approved to III) per 1705A.3.3.3.3.3.3.3.3.3.3.3.3.3.3.3.3.3.3.3
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• Mill certificates indicate in requirements. • Material sizes, types and b. Test unidentified material c. Examine seam welds of d. Verify and document stoconstruction documents. S/A2. HIGH-STRENGTH Test or Special Inspection of compliance conform to a DSA-approved documents.	AISI S240-20 Section A3 & A5, AISI S220-20 Section A4 & A6.* By special inspector or qualified technician whe performed off-site. unidentified materials Test LOR 2202A.1. DSA IR 17-3. Not applicable to cold-formed steel light-frame construction, except if trusses (1705A.2.4). IGH-STRENGTH BOLTS: Special Inspection Type Performed By Periodic SI Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and Signation in the proved documents. Ingl-strength bolts, nuts and washers. Test LOR Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 and N3.2; ROSC 2014 Section 1.5 & 2.1; DSA IR 17-8 BOSA IR 17-8 BOSA IR 17-9 BOSA I
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	al sizes, types and grades comply with requirements. unidentified materials Test LOR 202A.1. DSA IR 17-3. DSA IR 17-3. Not applicable to cold-formed steel light-frame construction, except from the steel fabrication per DSA-approved trusses (1705A.2.4). IGH-STRENGTH BOLTS: Special Inspection Type Periodic SI Table 1705A.2.1 Items 1a & 1b, 202A.1; AISC 360-16 Section A3 and M3.2; RCSC 2014 Section 9.1, DSA IR 17-8 & DSA IR 17-9. Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J.3.1, M2.5 & No.5, RCSC 2014 Section 9.1, DSA IR 17-9. Table 1705A.2.1 Items 2a & 2.1; DSA IR 17-9. Type Periodic SI Type
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S/A9. ANCHOR BOLTS A Test or Special Inspection a. Anchor Bolts and Anchor ame of Architect or Engineer	
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Test or Special Inspection a. Anchor Bolts and Anchor ame of Architect or Engineer	NOTION DULID AND ANOTHER RUDG.
a. Anchor Bolts and	Special Inspection Type Performed By Code References and Notes
ame of Architect or Engineer	
•	or Bolts and Anchor Hods Test LOR Sample and test anchor bolts and anchor rods not readily identifiable procedures noted in DSA IR 17-11.
-	procedure noted in Bort in 17 11.
-	tect or Engineer in general responsible charge:
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Signature of Architect or Struct	rchitect or Structural Engineer: Date:
Note: To facilitate DCA -1	litate DSA electronic mark-line and identification etamp application, DSA recommends expired using accuracy electronic and identification etamp application, DSA recommends expired using accuracy electronic and identification etamp application.
NOTE: 10 facilitate DSA electron	litate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.
	DSA STAMP

4. High-Strength Bolt Installation Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292

PSA 103-22: LIST OF REQUIRED VERIFIED REPORTS, CBC 2022

1. Soils Testing and Inspection: Geotechnical Verified Report Form DSA 293

2. Structural Testing and Inspection: Laboratory Verified Report Form DSA 291

3. Shop Welding Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292

4080 PLAZA GOLDORADO CIRCLE SUITE B. CAMERON PARK, CA 96882

STATE APPROVALS-SITE



- PORTER:



SIAIL AITROVALS-I C

IDENTIFICATION STAMP
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APP: 02-121214 PC

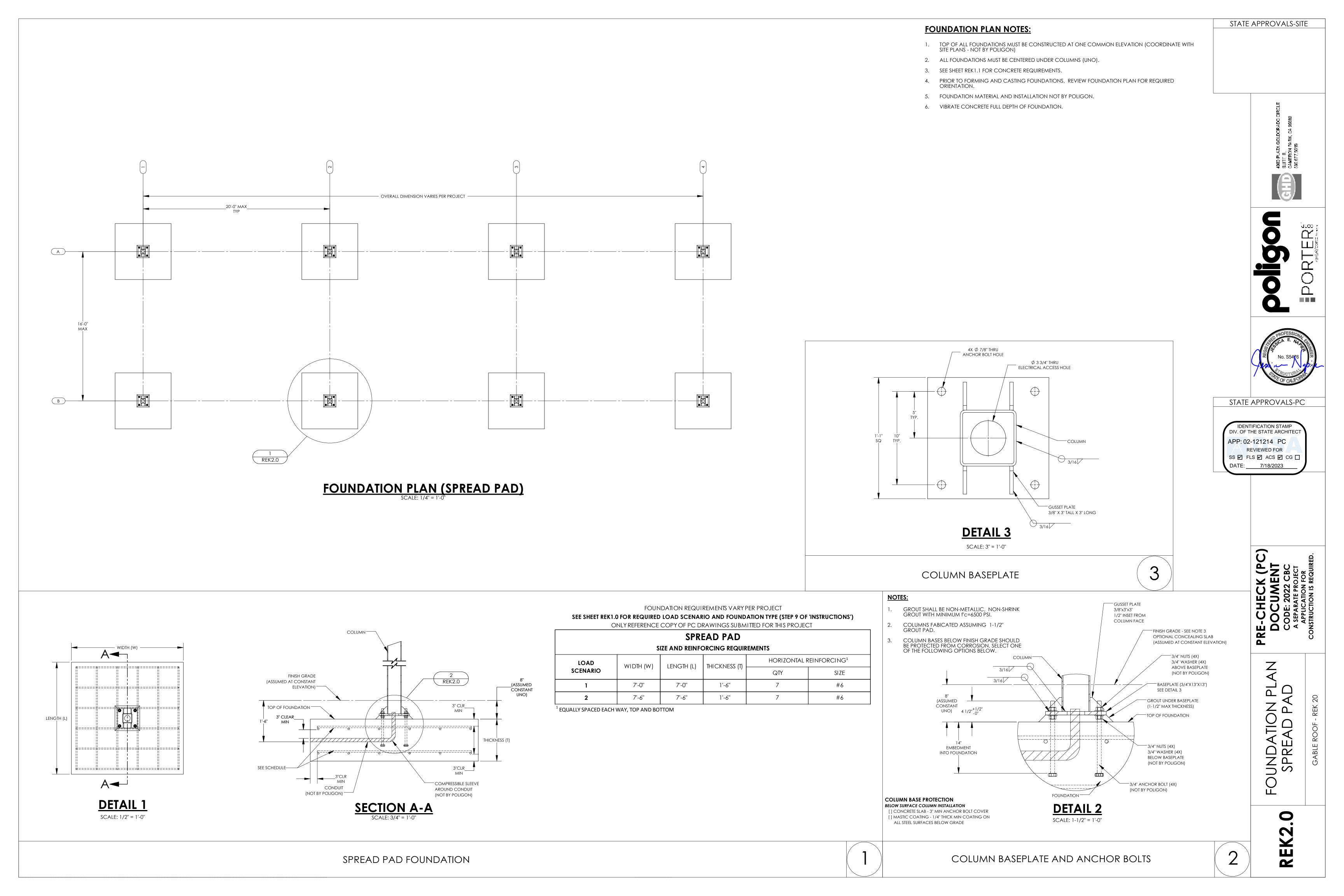
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SS FLS ACS CG

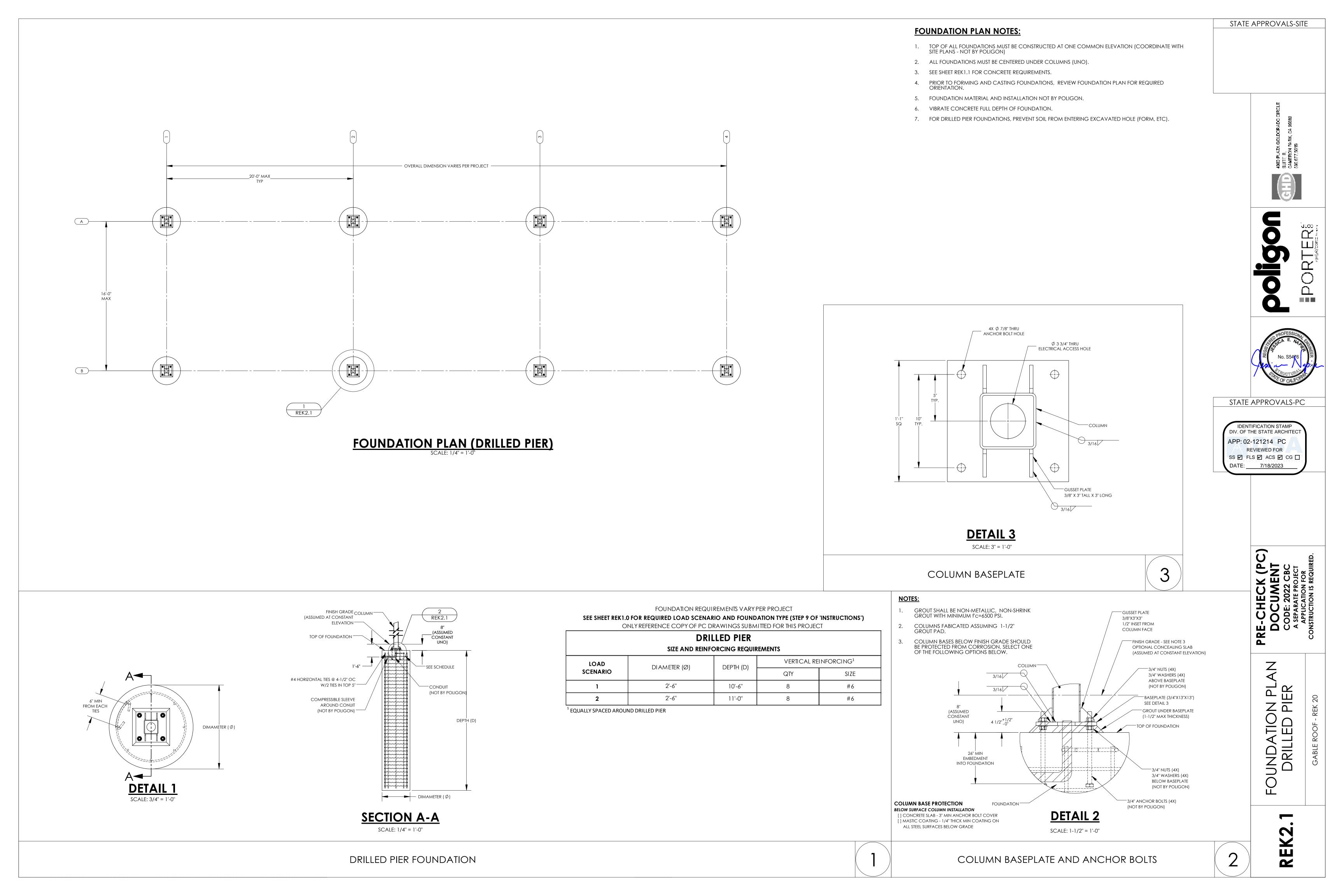
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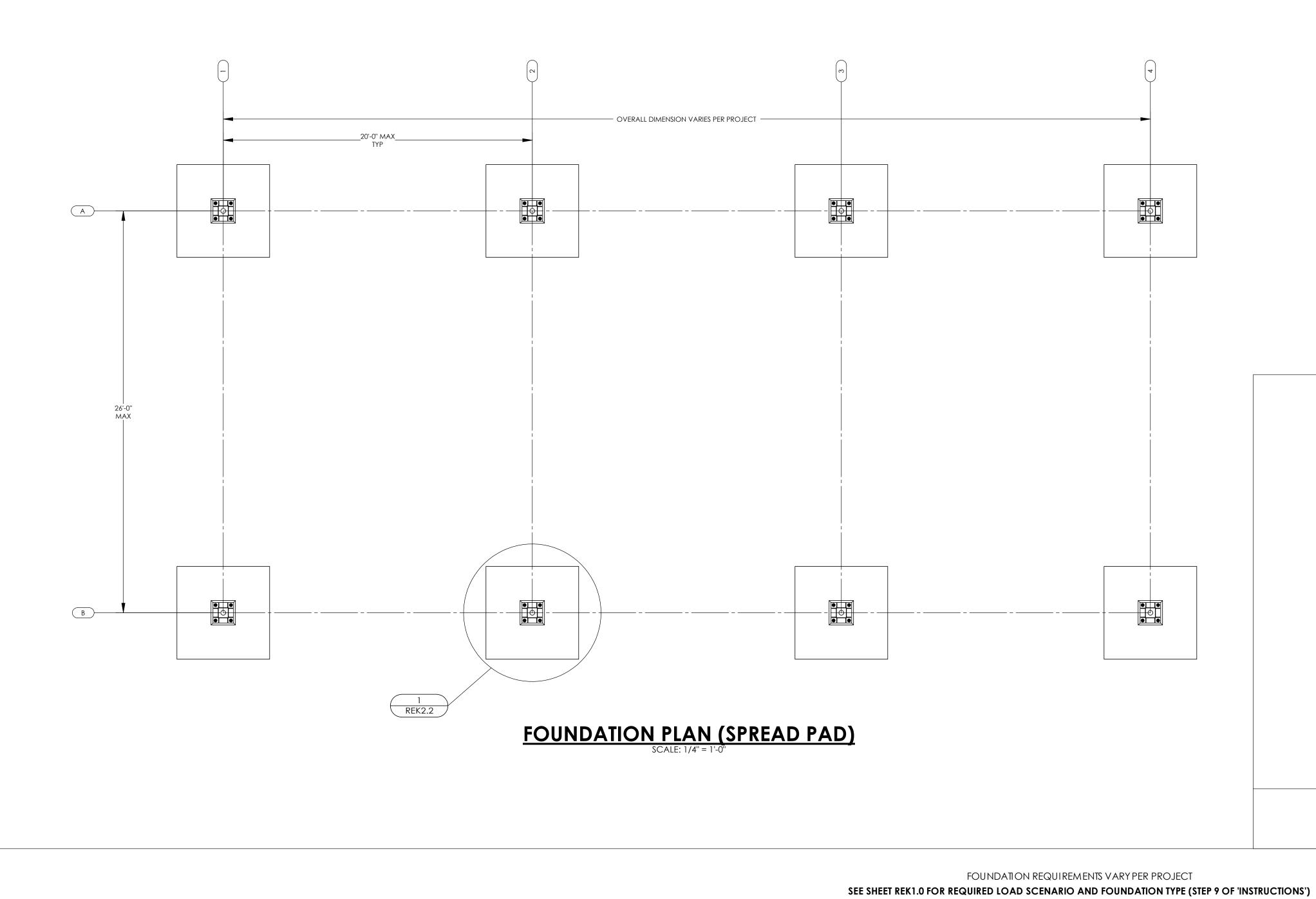
BCCHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT
APPLICATION FOR

OTES AND SPECIA INSPECTIONS

FK1







COLUMN-

2 / 9 / /

(NOT BY POLIGON)

FINISH GRADE

ELEVATION) —

(ASSUMED AT CONSTANT

TOP OF FOUNDATION —

3" CLR —

SEE SCHEDULE—

A

A

FOUNDATION PLAN NOTES:

TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLIGON)

Ø 3 3/4" THRU ELECTRICAL ACCESS HOLE

-COLUMN

GUSSET PLATE

COLUMN —

3/16

1/2" TYP ——

FOUNDATION —

3/8" X 3" TALL X 3-1/2" LONG

- 2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
- 3. SEE SHEET REK1.1 FOR CONCRETE REQUIREMENTS.
- PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED
- 5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLIGON.
- 6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

_ 4X Ø 1 1/8" THRU ANCHOR BOLT HOLE

DETAIL 3

SCALE: 3" = 1'-0"

COLUMN BASEPLATE

GROUT SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH MINIMUM I'C=6500 PSI.

COLUMN BASES BELOW FINISH GRADE SHOULD BE PROTECTED FROM CORROSION, SELECT ONE OF THE FOLLOWING OPTIONS BELOW.

8" (ASSUMED CONSTANT UNO)

EMBEDMENT

INTO FOUNDATION

COLUMN BASE PROTECTION

BELOW SURFACE COLUMN INSTALLATION

ALL STEEL SURFACES BELOW GRADE

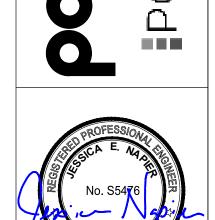
[] CONCRETE SLAB - 3" MIN ANCHOR BOLT COVER

[] MASTIC COATING - 1/4" THICK MIN COATING ON

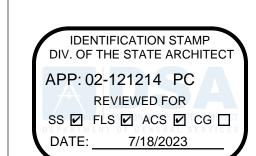
COLUMNS FABICATED ASSUMING 1-1/2" GROUT PAD.

STATE APPROVALS-SITE





STATE APPROVALS-PC



SPREAD PA

ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

SPREAD PAD

SIZE AND REINFORCING REQUIREMENTS					
LOAD	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING ¹	
SCENARIO				QTY	SIZE
1	8'-0''	8'-0''	1'-6"	8	#6
2	9'-0''	9'-0''	1'-6"	8	#6

¹ EQUALLY SPACED EACH WAY, TOP AND BOTTOM

(ASSUMED CONSTANT THICKNESS (T) COMPRESSIBLE SLEEVE AROUND CONDUIT (NOT BY POLIGON)

2 REK2.2

COLUMN BASEPLATE AND ANCHOR BOLTS

DETAIL 2SCALE: 1-1/2" = 1'-0"

GUSSET PLATE

3/8" X 3" TALL X 3-1/2" LONG

FINISH GRADE - SEE NOTE 3

- 1" NUTS (4X)

1" WASHERS (4X)

ABOVEBASEPLATE

(NOT BY POLIGON)

BASEPLATE (3/4"X16"X16")

GROUT UNDER BASEPLATE (1-1/2" MAX THICKNESS)
TOP OF FOUNDATION

SEE DETAIL 3

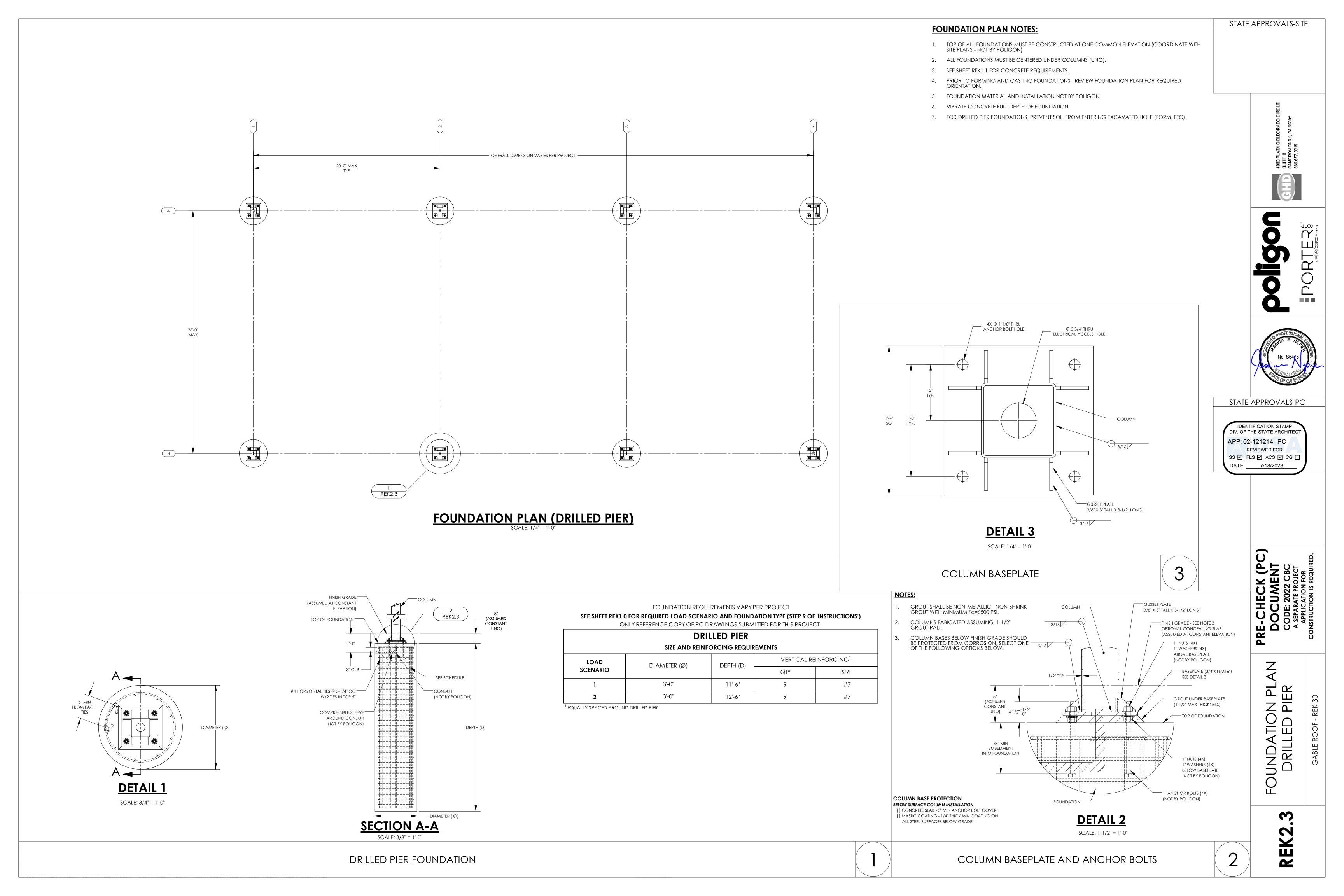
1" WASHERS (4X) BELOW BASEPLATE (NOT BY POLIGON)

(NOT BY POLIGON)

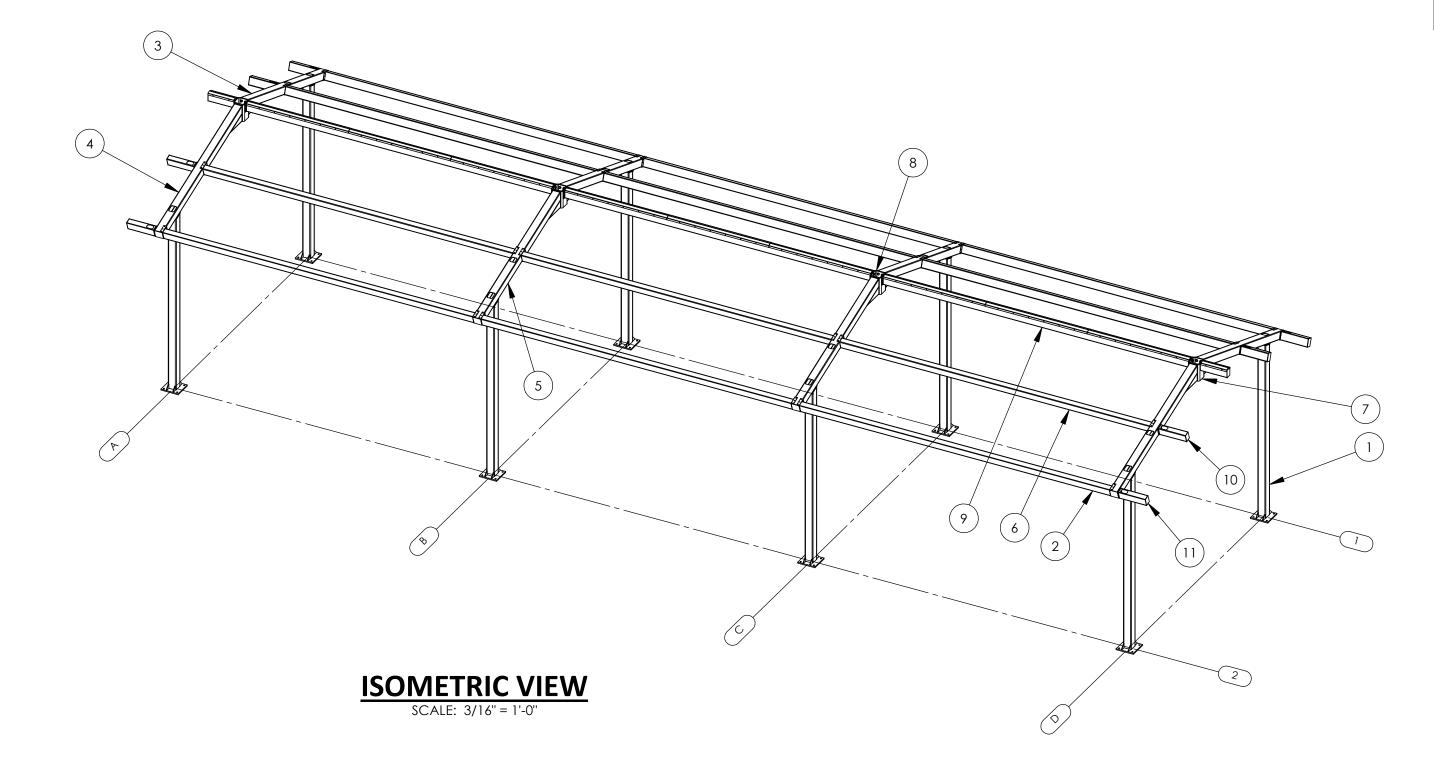
OPTIONAL CONCEALING SLAB (ASSUMED AT CONSTANT ELEVATION)

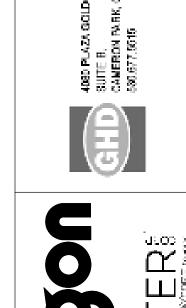
REK

SPREAD PAD FOUNDATION

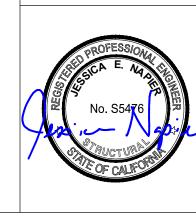










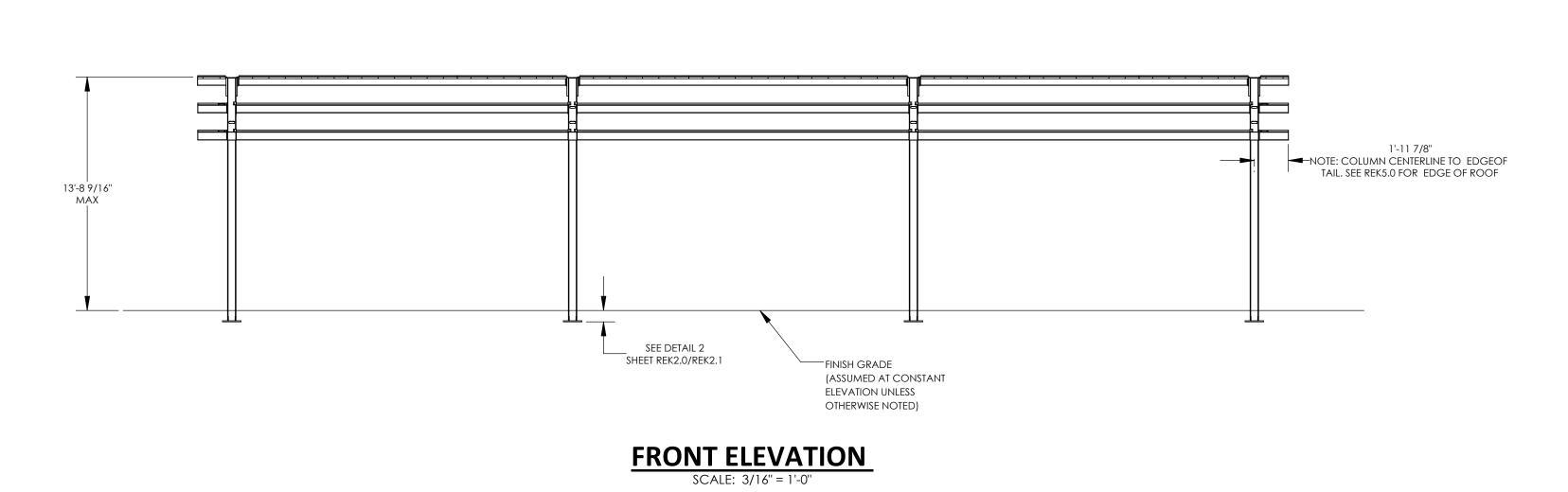


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APP: 02-121214 PC

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SS FLS ACS CG

DATE: 7/18/2023



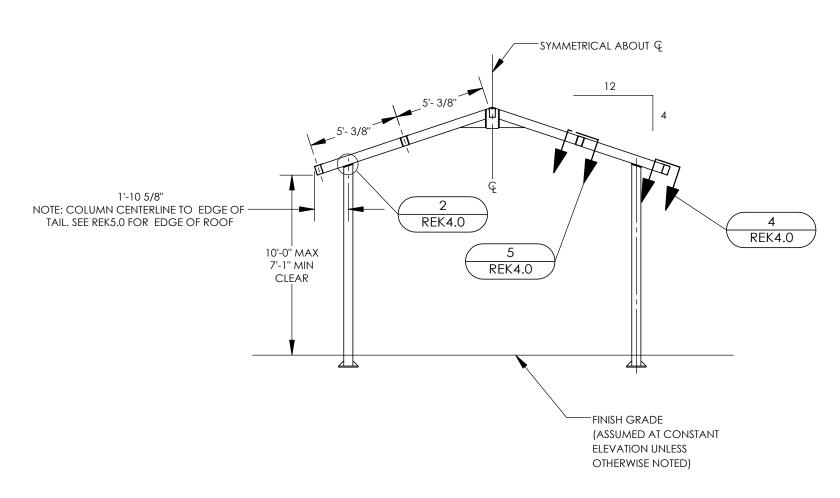
- OVERALL DIMENSION VARIES PER PROJECT —

PLAN VIEW

SCALE: 3/16" = 1'-0"

6 REK4.0

3 REK4.0

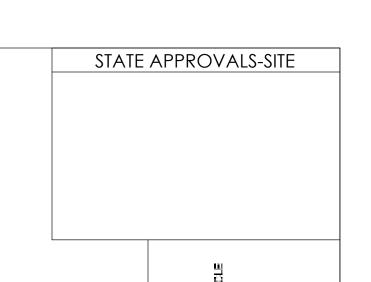


SCALE: 3/16" = 1'-0"

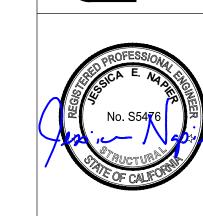
11	4	-	EAVE BEAM TAIL ASM	HSS6X4X3/16	16.49
10	4	-	PURLIN TAIL ASM	HSS6X4X3/16	22.66
9	3	-	RIDGE BEAM ASM	HSS6X4X1/8	163.15
8	2	-	MID C-TUBE ASM	HSS8X8X5/8	69.22
7	2	-	END C-TUBE ASM	HSS8X8X5/8	80.46
6	6	-	PURLIN ASM	HSS6X4X3/16	235.13
5	4	-	GABLE BEAM 3 ASM	HSS6X6X3/16	168.85
4	2	-	GABLE BEAM 2 ASM	HSS6X6X3/16	173.39
3	2	-	GABLE BEAM 1 ASM	HSS6X6X3/16	172.50
2	6	-	EAVE BEAM ASM	HSS6X4X1/8	161.42
1	8	-	COLUMN 1 ASM	HSS6X6X1/4	249.23
ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT

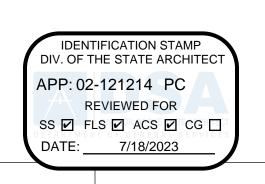
FRAMING PLAN

REK3.0









4 REK4.1 5 REK4.1 2 REK4.1

SCALE: 3/16" = 1'-0"

ISOMETRIC VIEW

SCALE: 3/16" = 1'-0"

1'-10 5/8" NOTE: COLUMN CENTERLINE TO EDGE OF — TAIL. SEE REK5.1 FOR EDGE OF ROOF

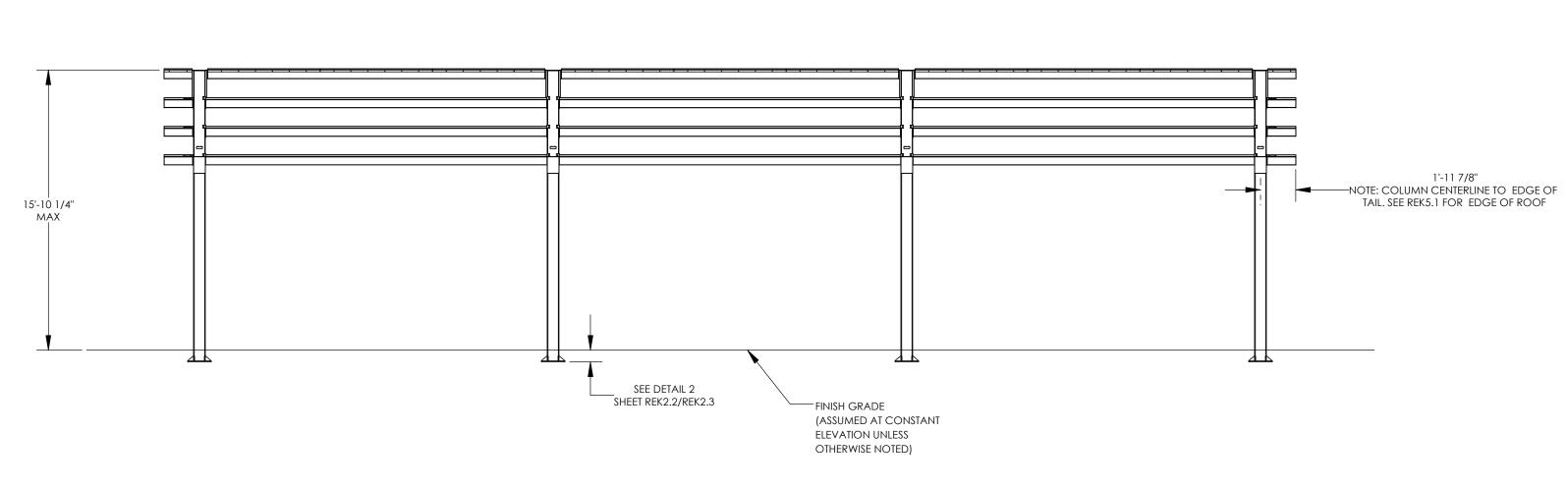
10'-0" MAX 7'-1" MIN CLEAR

11	12	-	PURLIN ASM	HSS6X4X3/16	235.90
10	4	-	EAVE BEAM TAIL ASM	HSS6X4X3/16	15.81
9	8	-	PURLIN TAIL ASM	HSS6X4X3/16	21.67
8	3	-	RIDGE BEAM ASM	HSS6X4X1/8	162.30
7	2	-	MID C-TUBE ASM	HSS10X6X5/8	100.89
6	2	-	END C-TUBE ASM	HSS10X6X5/8	114.86
5	4	-	GABLE BEAM 3 ASM	HSS12X8X3/16	428.21
4	2	-	GABLE BEAM 2 ASM	HSS12X8X3/16	433.72
3	2	-	GABLE BEAM 1 ASM	HSS12X8X3/16	433.72
2	6	-	EAVE BEAM ASM	HSS6X4X3/16	235.90
1	8	-	COLUMN 1 ASM	HSS8X8X1/4	347.39
ITEM	QTY.	PART NO.	DESCRIPTION	MATERIAL	WEIGHT

6 REK4.1 3 REK4.1 7 REK4.1 - OVERALL DIMENSION VARIES PER PROJECT -

PLAN VIEW

SCALE: 3/16" = 1'-0"

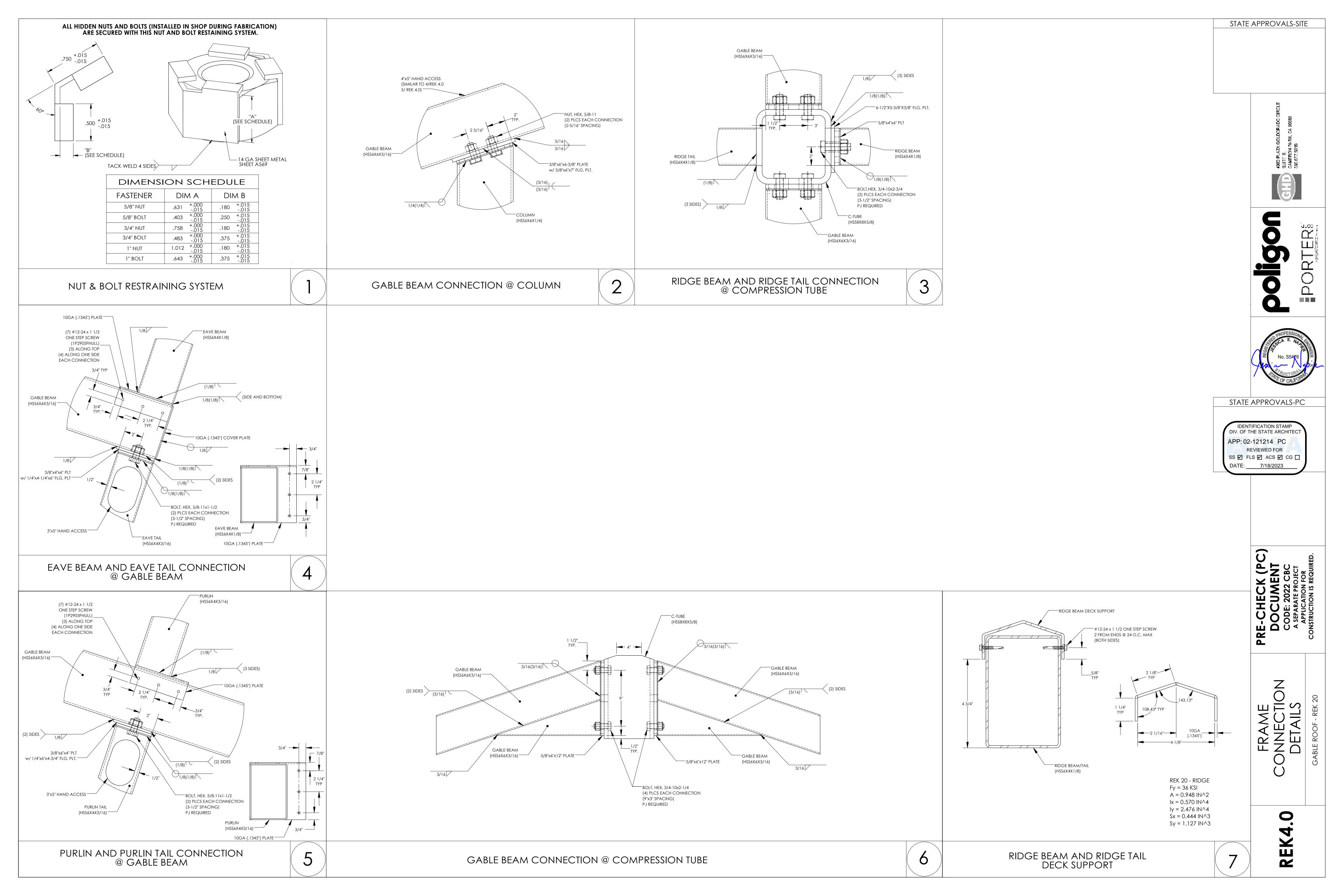


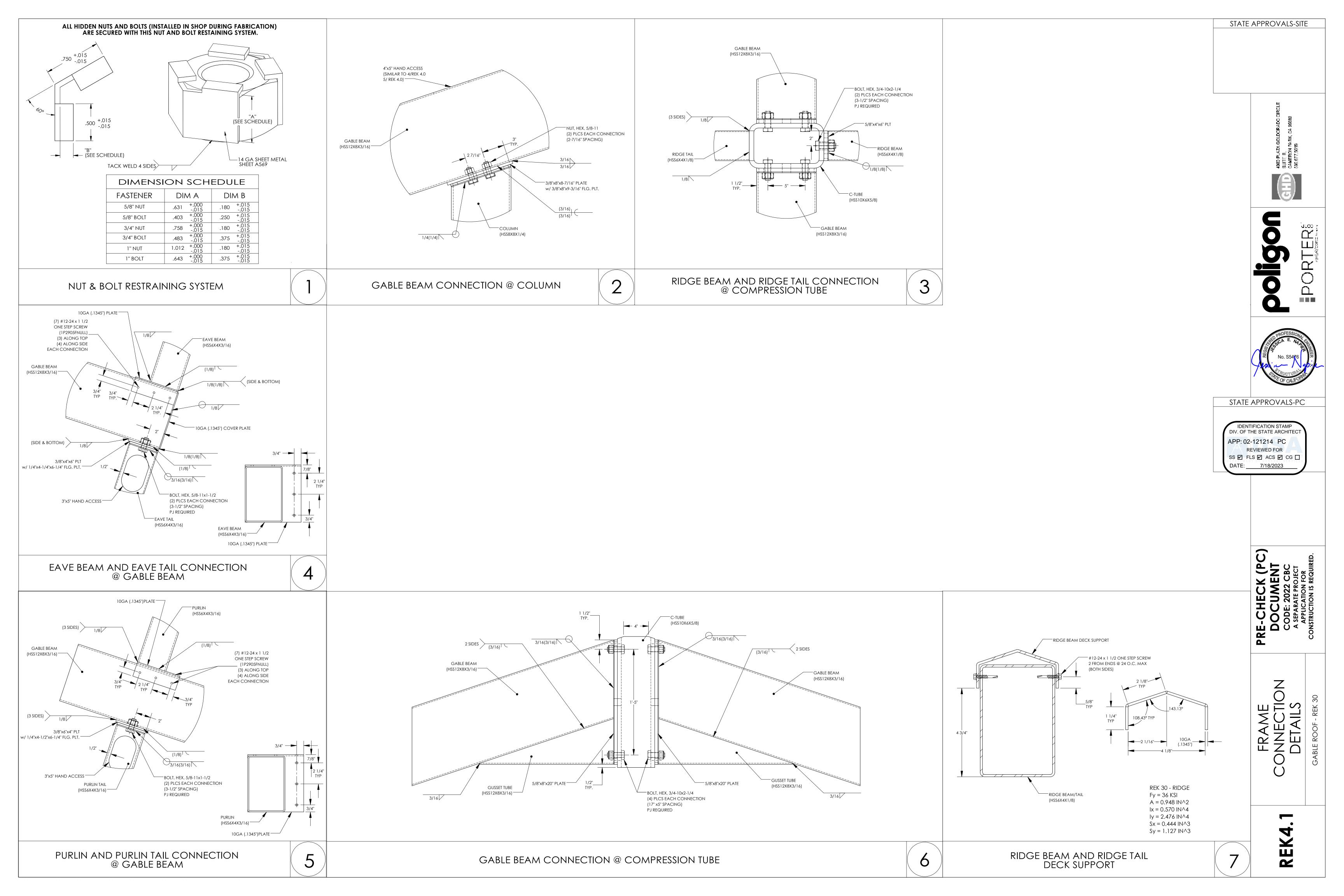
FRONT ELEVATION

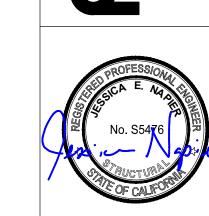
SCALE: 3/16" = 1'-0"

FRAMING

REK3.



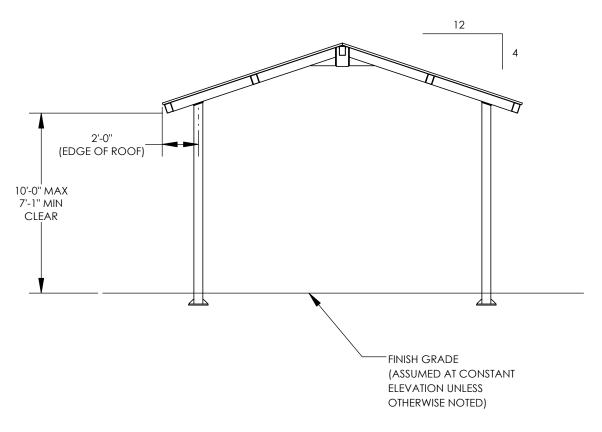




REK5.0

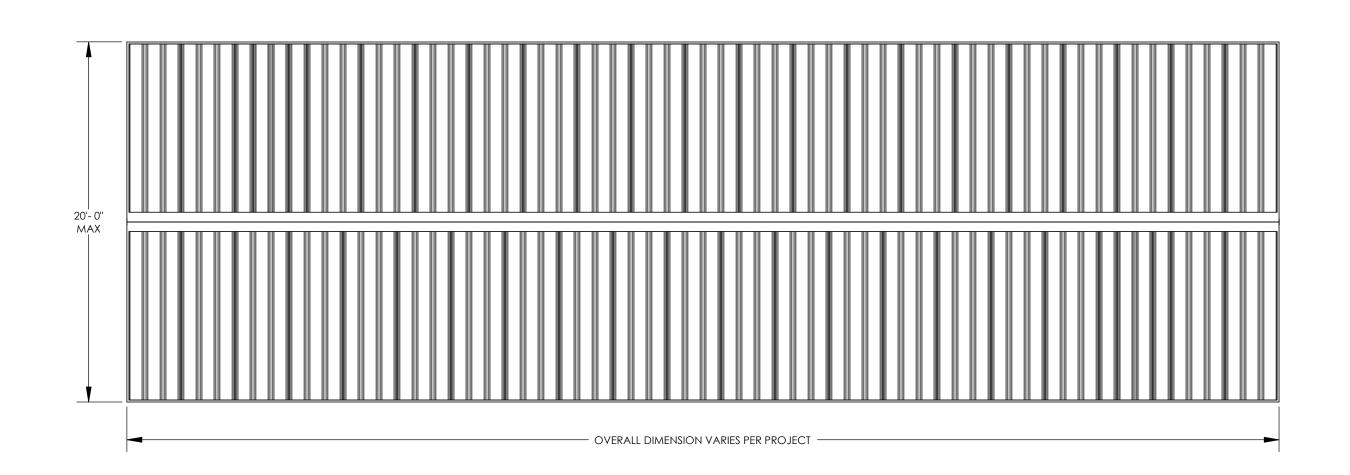
ARCHITECTURAL VIEWS

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 02-121214 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

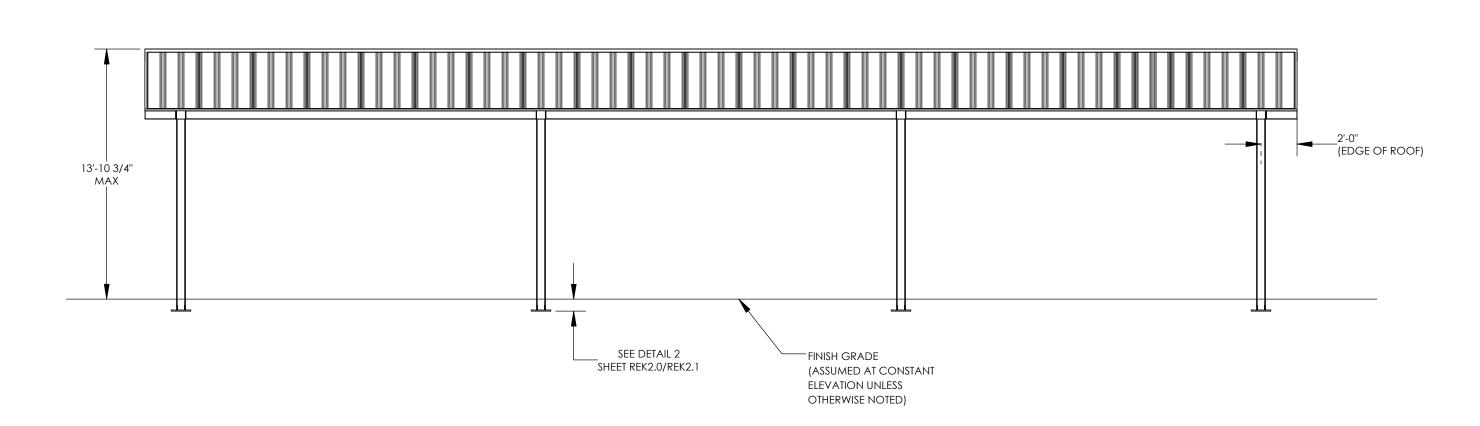


ISOMETRIC VIEW
SCALE: 3/16" = 1'-0"





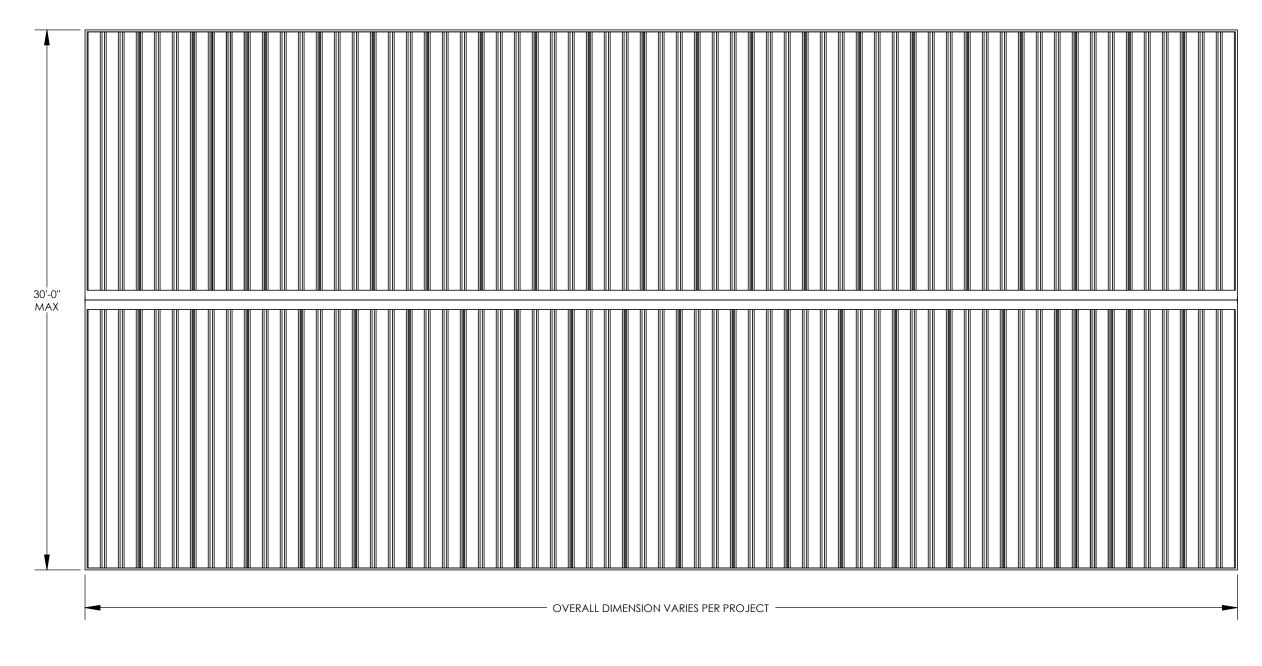
PLAN VIEW SCALE: 3/16" = 1'-0"



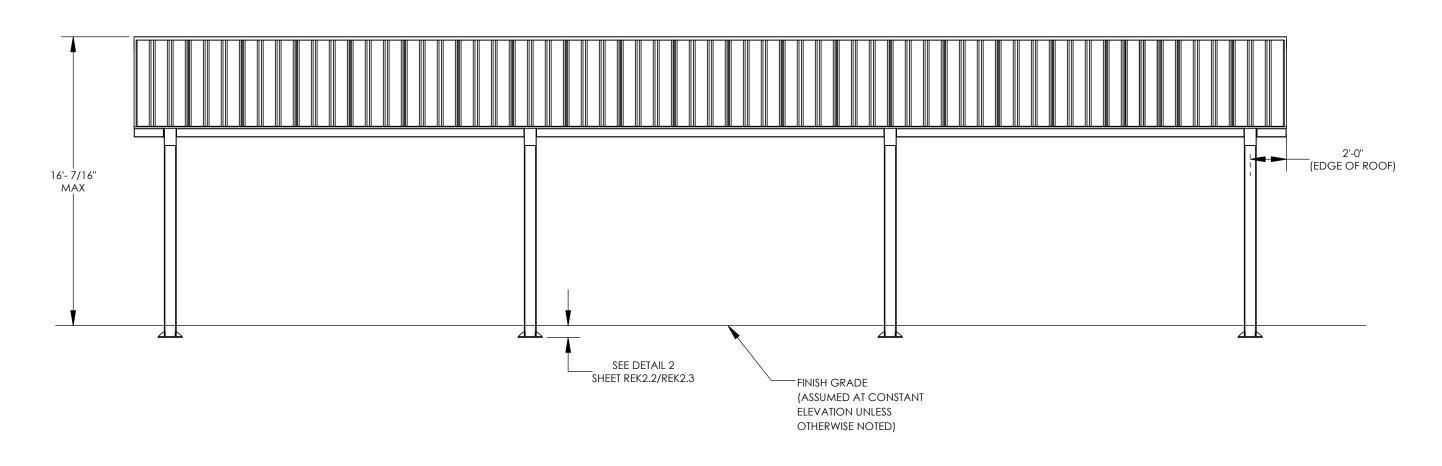
FRONT ELEVATION
SCALE: 3/16" = 1'-0"

ARCHITECTURAL VIEWS

REK5.

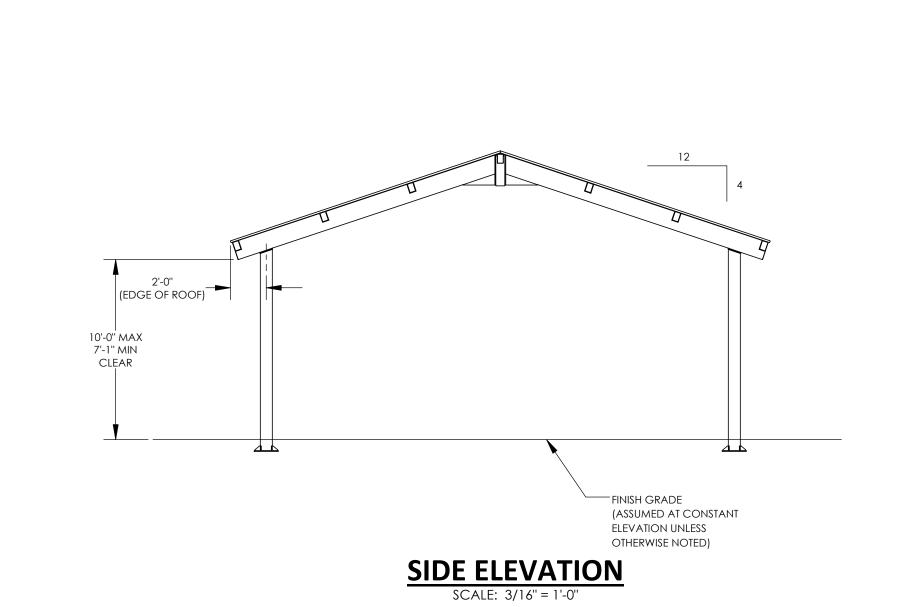


PLAN VIEW SCALE: 3/16" = 1'-0"

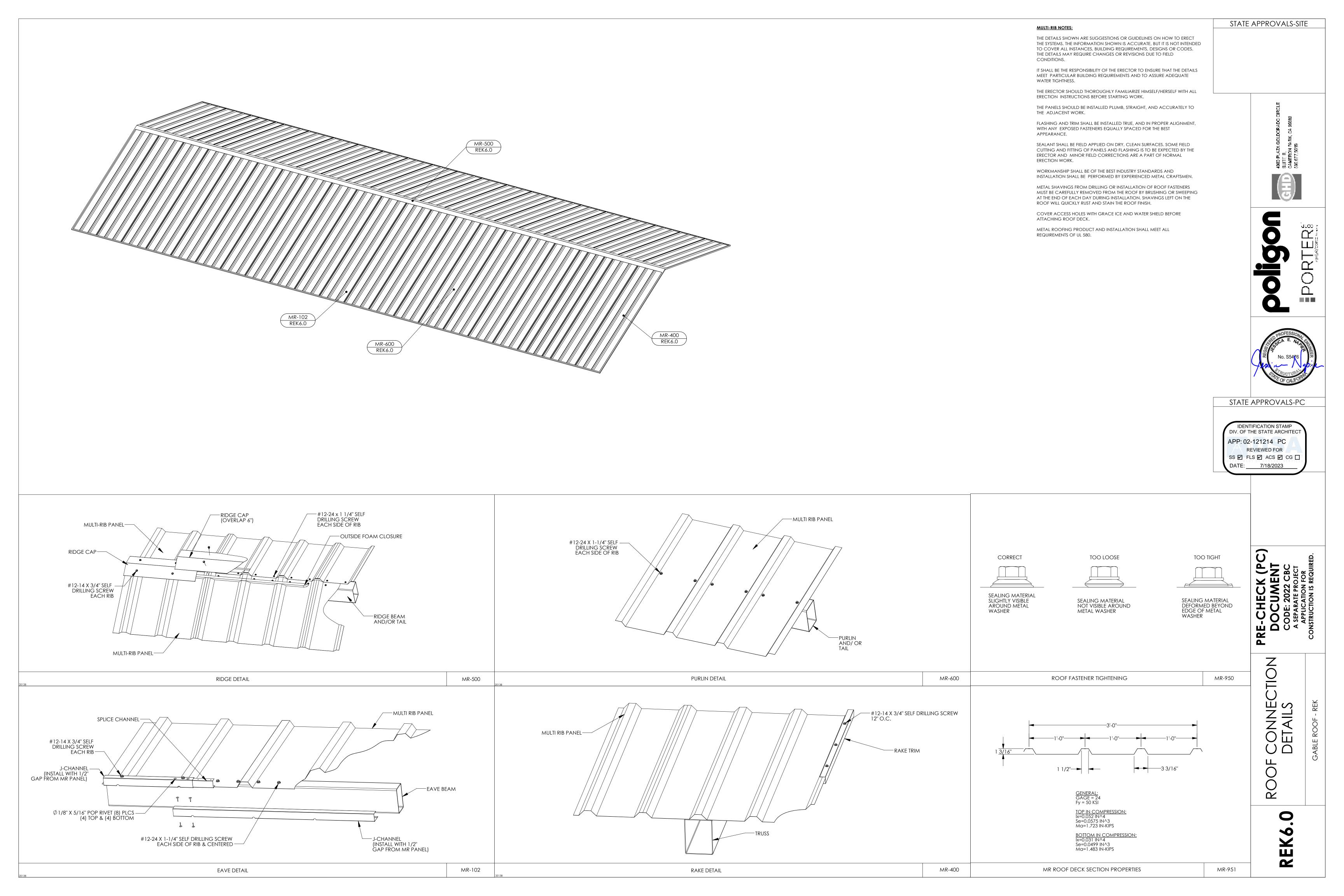


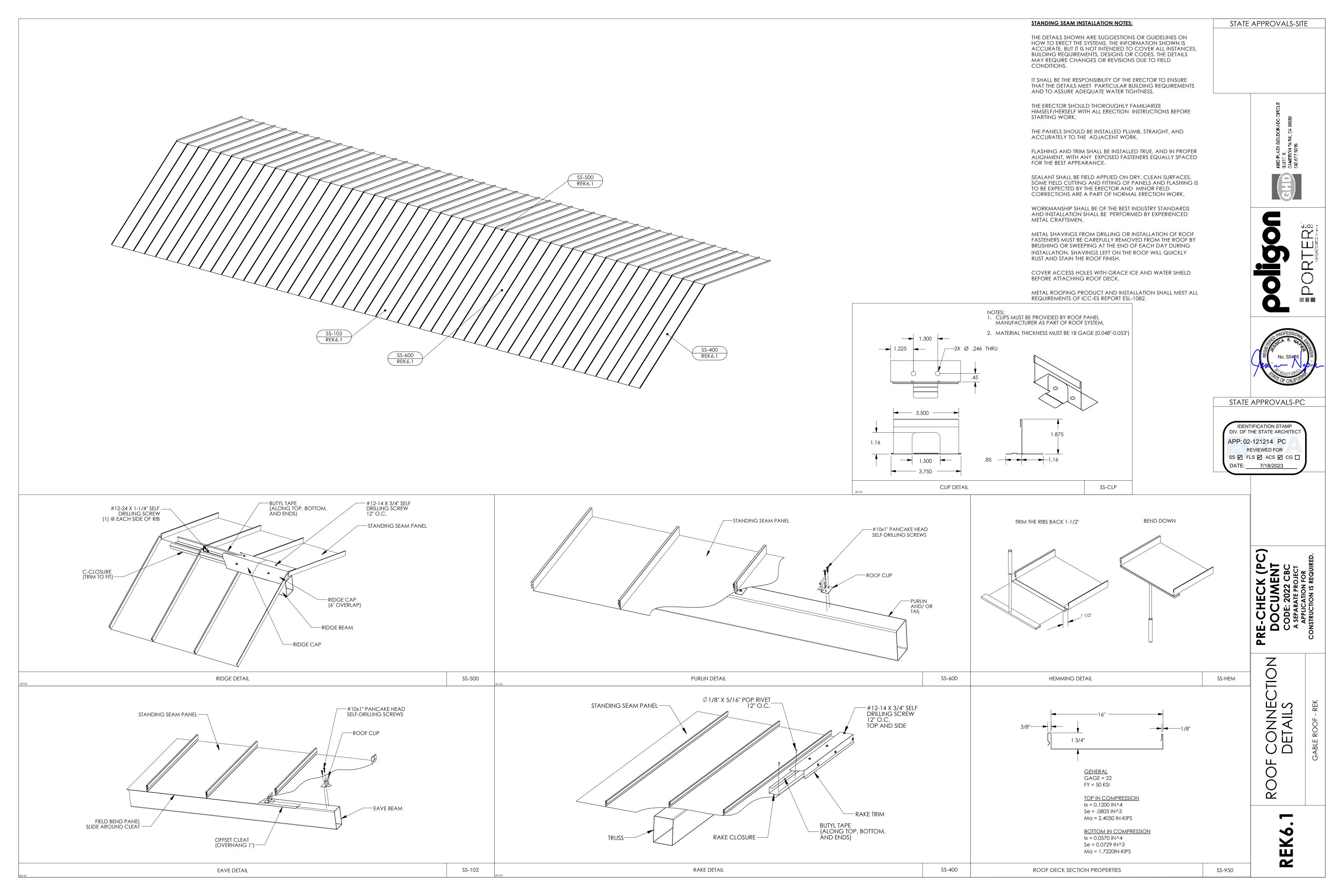
FRONT ELEVATION

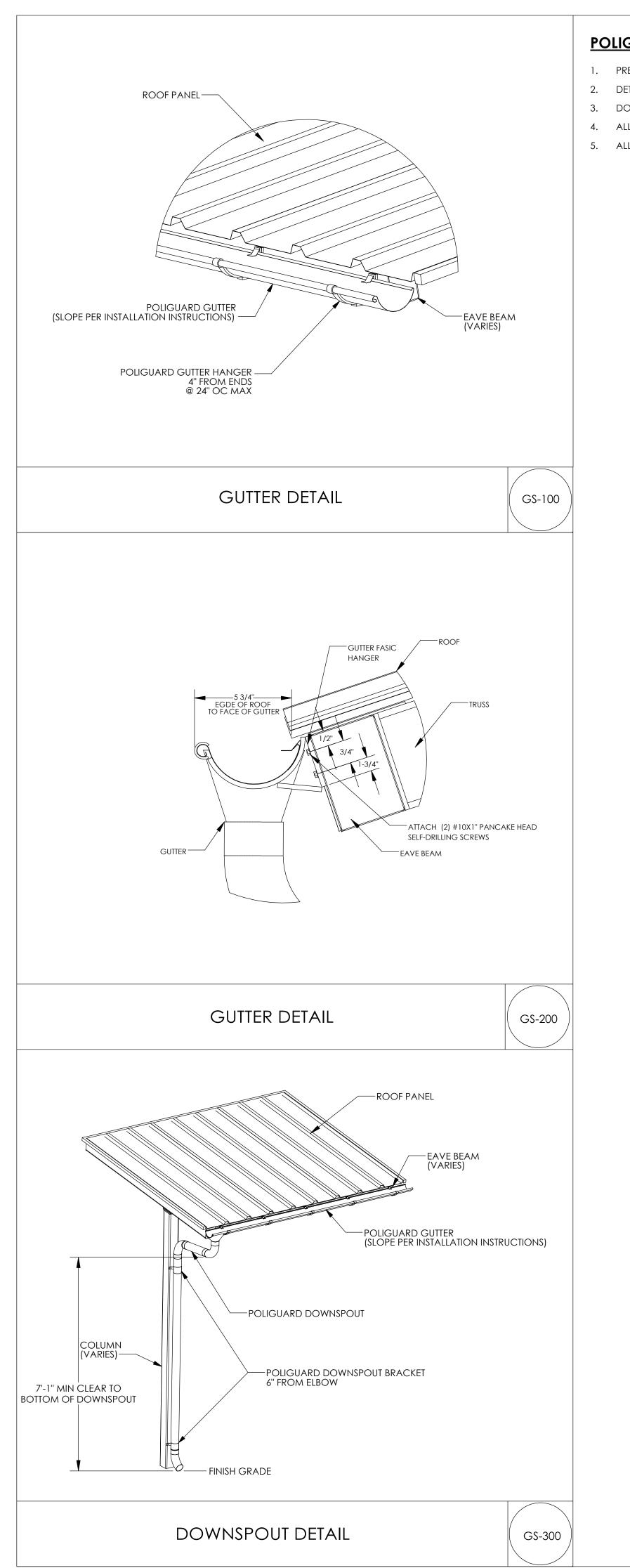
SCALE: 3/16" = 1'-0"



SCALE: 3/16" = 1'-0"







POLIGUARD GUTTER SYSTEM NOTES:

- 1. PREFABRICATED GUTTER SYSTEM IS ATTACHED TO THE STRUCTURE AFTER ROOF IS INSTALLED.
- 2. DETAILED INSTALLATION INSTRUCTIONS ARE SHIPPED WITH THE STRUCTURE.
- 3. DOWNSPOUTS REQUIRED AT EACH COLUMN.
- 4. ALL MATERIAL IS 24GA G90
- 5. ALL DOWNSPOTS AND SUTTERS ARE POWDER COATED WITH AN EPOXY PRIMED FINISH.

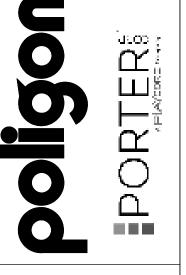
ELECTRICAL CUTOUT NOTES:

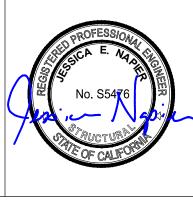
- 1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.
- 2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
- 3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.
- 4. ARCHITECTS REQUESTING CUTOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOUTS FOR APPROVAL AND FABRICATION.

4080 PLAZA GOLDORADO CIRCI SUITE B CAMERCIA PARK, DA 95682 530.677.5615

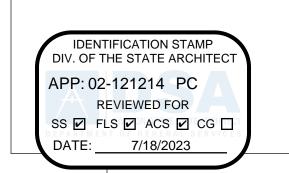
STATE APPROVALS-SITE







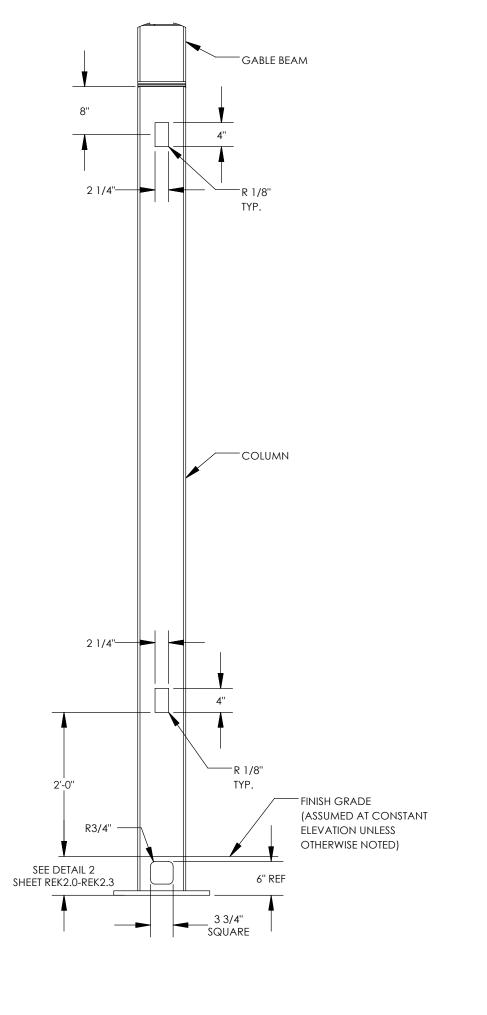
STATE APPROVALS-PC



RE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT

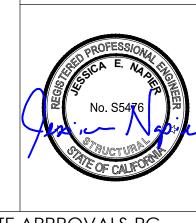
MISC DESIGN OPTIONS

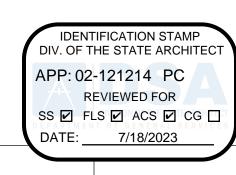
REK7.0



ELECTRICAL CUTOUT IN COLUMNS

EC-100





ELECTRICAL CUTOUTS

REK7

ELECTRICAL CUTOUT AND ACCESS INSTRUCTIONS

- IF 'YES' IS NOT SELECTED IN STEP 2 ON ORDER FORM, THEN THIS SHEET NEED

NOT BE INCLUDED IN SITE-SPECIFIC DRAWINGS

- ONLY COLUMNS ARE PERMITTED TO HAVE ELECTRICAL ACCESS

- THE COLUMN CUTOUTS ARE STATIC AND SHOWN IN THE 'MISC DESIGN OPTIONS SHEET'

- IDENTIFY THE COLUMNS WITH ELECTRICAL CUTOUTS BELOW (REFERENCE GRID LINES IN

ISOMETRIC FRAME VIEW TO THE RIGHT)

- STRUCTURES MAY BE LONGER OR SHORTER THAN THE ISOMETRIC FRAME VIEW SHOWN

- IF SITE-SPECIFIC STRUCTURE HAS A DIFFERENT NUMBER OF COLUMNS THAN ISOMETRIC SHOWN,

REFERENCE COLUMN A1 IN THE ISOMETRIC VIEW AND CONTINUE PATTERN TO FIT SITE-SPECIFIC LAYOUT

- IF NO COLUMNS ARE IDENTIFIED, POLIGON WILL ASSUME CUTOUTS ONLY IN COLUMN A1

- CONTACT POLIGON ENGINEERING FOR SPECIAL PROJECT SPECIFIC REQUIREMENTS

ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS

SPECIFIC MEMBERS

EXAMPLE:

ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS



SPECIFIC MEMBERS A1, B1, F1

