DESCRIPTION DEAD AND LIVE LOADS	DESIGN VALUES <sup>1</sup>		CTED	1 PROJECT INFORMA	ATION	
OOF LIVE LOAD	20 PSF	PROJECT NAME	<u>JILI_</u>	TTROJECT INTORMIZ	<u> </u>	
OOF DEAD LOAD (SUPERIMPOSED ON FRAME) <sup>2</sup>	LOAD SCENARIO= {1,2} DL= {3.5 PSF, 2.0 PSF}	SCHOOL DISTRICT USE AND OCCUPANCY				
ALLOWABLE SOIL PRESSURE 3,5		CLASSIFICATION		(15 SOET/BEE	<u> </u>	DCCUPANCY: A1, A2, A3, A4, A5, B, E  ERSON MIN FOR ANY A OCCUPANCY
SPREAD PAD	1500 PSF	OCCUPANT LOAD FACTOR		(15 5QFI/FER	20 SQFT/P	ERSON MAX FOR B or E OCCUPANCY
ERTICAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING)  ATERAL COHESION: DL + Lr + SEISMIC (CONCRETE FOOTING)	130 PSF	TOTAL ROOF AREA  NUMBER OF OCCUPANTS				
DRILLED PIER						
(IN FRICTION (DOWN): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.4 (IN FRICTION (UPLIFT): DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.1.5	167 PSF 83 PSF			TEP 2 DESIGN OPTIOI	<u>NS</u>	DEFAULT WEIGHT LODG
ATERAL BEARING: DL + Lr + SEISMIC (CONCRETE FOOTING) PER 1810A.3.3.2	100 PSF/FT	ROOF DECK	<u> </u>	MULTI-RIB (MR) STANDING SEAM (SS)		<b>DEFAULT</b> , WEIGHT 1.2 PS WEIGHT 1.8 PS
ROOF SNOW LOAD <sup>6</sup>		GUTTERS	<u> </u>	NO YES		SEE CWC7.0 FOR DETAIL
ROUND SNOW LOAD, Pg	10 PSF	ELECTRICAL ACCESS		NO		DEFAUL
SK CATEGORY  DOF SNOW LOAD: [] FLAT, Pf OR [] LOW SLOPE, Pm OR [X] SLOPED, Ps	III 11 PSF	LLECTRICAL ACCESS		YES 8'		SEE CWC7.1 FOR DETAIL  DEFAUL
OW ROOF SLOPE FACTOR, Cs	1.0	CLEAR HEIGHT	[]_	OTHER		10' MA
OW EXPOSURE FACTOR, Ce OW LOAD IMPORTANCE FACTOR, Is	1.2		STEP	3 SEISMIC ACCELERA	ATION	
ERMAL FACTOR, C†	1.2		Ss		(g)	
STANCE FROM ADJACENT STRUCTURE, Pg = 0 PSF	0 PSF 4 IN		\$1		(g)	
TANCE FROM ADJACENT STRUCTURE, Pg > 0 PSF	20 FT			TED 4 OFICE AS SEC.	NIC .	
FLOOD DESIGN	0 PSF	0.000 <\$s <= 1.406		<u>tep 4 seismic regioi</u>   white	<u>N3</u>	3.5 PSF MAX DEAD LOAI
DOD HAZARD AREA	NO	1.406 < Ss <= 2.063		GREEN		2.0 PSF MAX DEAD LOAI
WIND DESIGN <sup>4</sup>			CTED	E TOTAL BOOK DEAD	1040	
SIC WIND SPEED (3 SECOND GUST), Vult	115 MPH		<u> </u>	5 TOTAL ROOF DEAD	LOAD	
(POSURE CATEGORY DPOGRAPHIC FACTOR, Kzt (1 MINIMUM)	C 1	ROOF DECK		PSF		SEE STEP 2' 'ROOF DECK' FOR WEIGHT
TERNAL PRESSURE COEFFICIENT, GCpi (IF APPLICABLE)	0.0					
EAR WIND FLOW SSTRUCTED WIND FLOW	YES YES	COLLATERAL		PSF		LIGHTING , FIRE SUPPRESSION, ETC
SEISMIC DESIGN <sup>4</sup> TERAL FORCE-RESISTING SYSTEM	STEEL ORDINARY CANTILEVER	TOTAL		PSF		ADD 'ROOF DECK' AND 'COLLATERA
	COLUMN SYSTEM EQUIVALENT LATERAL FORCE					
ALYSIS PROCEDURE SMIC DESIGN CATEGORY (SDC)	PROCEDURE F		<u>z</u> .	TEP 6 LOAD SCENAR	<u>.IO</u>	
SMIC IMPORTANCE FACTOR, Ie	1.25	WHITE		TOTAL ROOF DEAD LOA	\D <= 3.5 PSF	[ ] LOAD SCENARIO
SIGN BASE SHEAR, V SMIC RESPONSE COEFFICIENT, Cs	Cs x W LOAD SCENARIO = {1,2}	GREEN		TOTAL ROOF DEAD LOA	AD < 2.0 PSF	[ ] LOAD SCENARIO
SPONSE MODIFICATION FACTOR, R	Cs = {1.13,1.65} 1.25			STEP 7 PC STRUCTURE	 E	
CLASS <sup>7</sup>	E		F WIDTH <= 10		[ ] CWC 10	
DUNDANCY FACTOR, p  APPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Ss - USED TO	1.3 LOAD SCENARIO = {1,2}	10 < RC	OOF WIDTH <= 15		[ ] CWC 15	
TERMINE Cs  ORT-PERIOD SITE COEFFICIENT, Fa	Ss = {1.406, 2.063}		(	STEP 8 STRUCTURE SIZ	<u></u> <u>′E</u>	
SIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds - USED TO TERMINE Cs	LOAD SCENARIO = {1,2} Sds (MAX) = {1.125, 1.650}			CWC 10		CWC 15
PPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, \$1	LOAD SCENARIO = {1,2}		[ ] 10'	DEFAULT	[ ] 15'	DEFAULT
NG-PERIOD SITE COEFFICIENT, FV	S1 = {0.844, 1.07} 2.0	ROOF WIDTH	[ ]	OTHER 6' MIN; 10' MAX		HER 10'-6" MIN; 15' MAX
SIGN SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, Sd1	LOAD SCENARIO = {1,2} Sd1 = {1.125, 1.427}			OTTER 6 MIN, 10 MAX		ILK 10-0 MIIN, 10 MAX
DRIZONTAL OR VERTICAL IRREGULARITIES TYPE(S)	NONE		[ ] 44'	2 BAYS	[ ] 36'	2 BAYS
1. IF SITE-SPECIFIC DESIGN CRITERIA ARE OUTSIDE THE LIMITS OF THESE PC DRAWING TO SEE IF AN ENGINEERING LETTER, SUPPLEMENTAL DRAWINGS, AND/OR CALCU	GS, CONTACT POLIGON ENGINEERING LATIONS COULD BE SUBMITTED FOR A	DOOF LENGTH	[ ] 64'	3 BAYS	[ ] 52'	3 BAYS
SITE-SPECIFIC SOLUTION. ANY SITE-SPECIFIC DEVIATION FROM THIS PC MAY NOT OVER- THE- COUNTER	BE SUBMITTED TO DSA AS AN	ROOF LENGTH				
2. STRUCTURE IS NOT DESIGNED TO SUPPORT SOLAR PANELS. STRUCTURE IS NOT DE SPRINKLER SYSTEMS IN LOAD SCENARIO 2 REGIONS.	SIGNED TO SUPPORT		[ ] 84'	4 BAYS	[ ] 68'	4 BAYS
3. GEOHAZARD REPORTS ARE REQUIRED IF THE AREA COVERED UNDER THE ROOF	EXCEEDS 4000 SQ FT OR IS LOCATED		[]	_ OTHER	[ ]OTH	HER
WITHIN STATE OR LOCAL GEOLOGIC HAZARD ZONE. VERIFY SUBMITTAL AND APPCGS PRIOR TO DSA SITE APPLICATION.	ROVAL OF A GEOHAZARD REPORT BY		ST	TEP 9 FOUNDATION TY	<u>/PE</u>	
I. STRUCTURAL SEPERATION BETWEEN ADJACENT STRUCTURES: CWC10= 2.75" CW STRUCTURAL SEPARATION BETWEEN EXISTING STRUCTURES: CWC10= 3.75" CWC		FOUNATION TYPE [ 1.SP	PREAD PAD	VC 10  [ ] DRILLED PIER	[ ] SPREAD PA	CWC 15
5. WHEN PLACING MULTIPLE CANOPIES WITH PIER FOOTINGS ADJACENT TO ONE A	NOTHER, THE DESIGN MAY	[ 10]				, 1
REQUIRE AN ANALYSIS OF GROUP EFFECTS ON THE FOUNDATIONS. THE MINIMUM PIERS IS EIGHT TIMES PIER DIAMETER WITHOUT AN ACCOMPANYING ENGINEERIN	N CLEARANCE BETWEEN CENTER OF G LETTER.	CWC 10	STEP 1	10 FOUNDATION SUM	MARY CWO	^ 15
S. SITE APPLICATION DESIGN PROFESSIONAL AND DSA REVIEWER SHALL VERIFY THE FEET FROM ANY ADJACENT HIGHER STRUCTURE IF GROUND SNOW LOAD IS GRE			[ ] LOAD SC	ZENARIO I	AD SCENARIO 1	[ ] LOAD SCENARIO 1
7. DESIGN COMPLIES WITH THE CONDITIONS OF EXCEPTION 1 OF ASCE 7-16 SECTION		SPREAD PAD	DRILLED	PIER SPF	READ PAD	DRILLED PIER
3. APPROVED FIRE APPARATUS ACCESS ROADS SHALL EXTEND TO WITHIN 150 FEET	·	[ ] LOAD SCENARIO 2	[ ] LOAD SC	CENARIO 2	AD SCENARIO 2	[ ] LOAD SCENARIO 2
OF THE STRUCTURE PER CFC 503.1.1.		SPREAD PAD	DRILLED	PIER SPF	READ PAD	DRILLED PIER
ARCHITECTURAL REQUIREMENTS:  DESCRIPTION	DESIGN VALUES					
E OF CONSTRUCTION	II B	BASE FRAME	CWC 10 C	STEP 11 SHEET INDEX SHEET INDEX		CWC 15 SHEET INDEX
MBER OF STORIES	1		MR MR	SS SS	MR	SS SS
SPRINKLER SYSTEM  ELATED BLULDING CODES AND STANDARDS.	NOT BY POLIGON					
ELATED BUILDING CODES AND STANDARDS:		FOUNDATION TYPE SPREAD P	PAD DRILLED SPR	READ PAD DRILLED PIER	SPREAD DRILL PAD PIER	ED SPREAD DRILLED PIER
TLE 24 CODES:  022 California Administrative Code (CAC)(Part 1, Title 24, CCR)						
<b>022 California Building Code (CBC), Volumes 1 and 2</b> (Part 2, Title 24, CCR) (2021 International Building Code with 2022 California amendments)		SELECT ONE [ ]			1 1 1	1 1 1 1
<b>022 California Electrical Code</b>		ORDER FORM CWC1.	.0 CWC1.0 C	CWC1.0 CWC1.0	CWC1.0 CWC	1.0 CWC1.0 CWC1.0
022 California Mechanical Code (CMC)(Part 4, Titlé 24, CCR) (2021 Uniform Mechanical Code with 2022 California amendments)		NOTES AND SPECIAL INSPECTIONS CWC1.		CWC1.1 CWC1.1	CWC1.1 CWC	
<b>022 California Plumbing Code (CPC)</b> (Part 5, Title 24, CCR) (2021 Uniform Plumbing Code with 2022 California amendments)		FOUNDATION PLAN CWC2.	.0 CWC2.1 C	CWC2.0 CWC2.1	CWC2.2 CWC	2.3 CWC2.2 CWC2.3
		FRAMING PLAN CWC3.	.0 CWC3.0 C	CWC3.0 CWC3.0	CWC3.1 CWC	3.1 CWC3.1 CWC3.1
2022 California Fire Code (CFC)		EDANAE CONINIECTION				
2022 California Fire Code (CFC)		FRAME CONNECTION DETAILS CWC4.		CWC4.0 CWC4.0	CWC4.1 CWC	
2022 California Fire Code (CFC)		ARCHITECTURAL VIEWS CWC5.	.0 CWC5.0 C	CWC5.0 CWC5.0	CWC5.1 CWC	5.1 CWC5.1 CWC5.1
2022 California Energy Code		DETAILS CWC4.	.0 CWC5.0 C			5.1 CWC5.1 CWC5.1 6.0 CWC6.1 CWC6.1

**2022 CBC**, CHAPTER **35 2022 CFC**, CHAPTER **80** 

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED

CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THIS STRUCTURE TO COMPLY WITH A WIDE VARIETY

STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF TUBULAR STEEL MEMBERS SUPPORTED ON

**SCOPE OF WORK NARRATIVE:** 

OF PROJECT SITES AND LOADING REQUIREMENTS.

	DESIGN VALUES 1										
S	20 PSF	STEP 1 PROJECT INFORMATION									NERAL PROJECT INFORMATION PROJECT NAME AND SCHOOL DIS <sup>*</sup>
	PROJECT NAME  SCHOOL DISTRICT							- IDENTIFY U - THE USE	JSE AND OCCUPANCY CLASSIFICA E AND OCCUPANCY DETERMINE T		
	USE AND OCCUPANCY CLASSIFICATION (PROPOSED OCCUPANCY: A1, A2, A3, A4, A5, B, E						I, A2, A3, A4, A5, B, E)	- IDENTIFY T	AXIMUM SQUARE FOOTAGE IS ALS THE OCCUPANT LOAD PER TABLE 1		
URE <sup>3,5</sup>		OCCUPANT LOAD FACTOR			(15 SQFT/PERS	SON MAX; 5	SQFT/PERSO	N MIN FOR	ANY A OCCUPANCY R B or E OCCUPANCY)	- IDENTIFY E	「OTAL ROOF AREA WHICH SHALL N EXPECTED NUMBER OF OCCUPAN' ROOF AREA DIVIDED BY OCCUPA
OTING)	1500 PSF	TOTAL ROOF AREA					3QFI/FERSO	N MAX FOR	R B OF E OCCUPANCE)		SIGN OPTIONS
DOTING)	130 PSF	NUMBER OF OCCUPANTS								- SELECT RC	OOF DECK FOR YOUR PROJECT
E FOOTING) PER 1810A.3.3.1.4	167 PSF			STEP 2 DE	SIGN OPTION	<u>1S</u>				- "SS" REF	EPRESENTS MCELROY METAL "MUL" PRESENTS MCELROY METAL "MEDA HETHER GUTTERS AND DOWNSPOL
FOOTING) PER 1810A.3.3.1.5 DTING) PER 1810A.3.3.2	83 PSF 100 PSF/FT	ROOF DECK		[ ] MULTI-RIB (I				DEI	FAULT, WEIGHT 1.2 PSF	- IF "YES". -SELECT WH	', THEN INCLUDE SHEET CWC7.0 IN HETHER ELECTRICAL CUTOUTS ARE
JIING) FER TOTUA.3.3.2	100 F3F/F1			[ ] STANDING	SEAM (SS)				WEIGHT 1.8 PSF <b>DEFAULT</b>	- SHEET C	CWC7.0 SHOWS ELECTRICAL CUTC CWC7.1 HAS INSTRUCTIONS AND S CWC7.1 MUST BE FILLED OUT IN TH
3	10 PSF	GUTTERS		[ ] YES [ ] NO				SEE	CWC7.0 FOR DETAILS  DEFAULT	- IF NOTH (SFF 'FR)	HING IS FILLED IN ON CWC7.1, POI 'AMING PLAN' FOR REFERENCE)
	III	ELECTRICAL ACCESS		[ ] YES				SEE	CWC7.1 FOR DETAILS	- SELECT CL - MIN 7'	LEAR HEIGHT (SEE 'ARCHITECTURAI '-1''; MAX 10'-0''
n OR [X] SLOPED, Ps	11 PSF	CLEAR HEIGHT		[ ] 8' [ ] OTHEF	)				DEFAULT 10' MAX	- IF NOTHIN	IG IS SELECTED, POLIGON WILL ASS
	1.0								10 170 01		ntify the Ss & S1 ACCELERATION (
	1.1			SS	C ACCELERA					- Ss & S1 VA	ALUE DETERMINES THE REQUIRED SI ALUE DEPENDS ON PROJECT'S GEO
	0 PSF					(g) (g)				- FIND Ss	s & \$1 VALUES FOR YOUR PROJECT s & \$1 VALUES FOR YOUR PROJECT NOT APPROVED FOR \$s VALUES G
	4 IN 20 FT									ADDITION	NAL OPTIONS)
	0 PSF		0.044		SMIC REGION	<u>1S</u>	1	0.5.			NTIFY THE SEISMIC REGION FOR YC
	NO		<=0.844 <=1.070	[ ] WHITE [ ] GREEN					PSF MAX DEAD LOAD	- THE REGIC - THE SEISM	ONS ARE DEPENDANT ON THE SS & IIC REGION DICTATES THE MAXIMU
										İ	NTIFY THE ROOF DEAD LOAD FOR '
	115 MPH			STEP 5 TOTAL	ROOF DEAD	<u>LOAD</u>				- THE COLL	F DECK DEAD LOAD WILL ALWAYS ATERAL LOAD REPRESENTS ADDITION
	C 1	ROOF DECK		·	PSF		SEE ST	TEP 2' 'ROO	f DECK' FOR WEIGHTS	- CUT SHEET	OOF DEAD LOAD MUST BE LESS THA TS OF ANY BOARDS, BOXES AND E AND DIMENSIONS ARE REQUIRED
LE)	0.0										NTIFY THE LOAD SCENARIO
	YES YES	COLLATERAL		·	PSF		LIC	GHTING , FI	re suppression, etc.	-	CE THE STEP 4 COLOR AND SELECT ENARIOS HAVE NO IMPACT ON FR
											NTIFY PC STRUCTURE
	STEEL ORDINARY CANTILEVER	TOTAL		·	PSF		ADD	'ROOF DEC	CK' AND 'COLLATERAL'		OTHS UP TO 10' WIDE USE THE "CWO OTHS UP TO 15' WIDE USE THE "CWO
	COLUMN SYSTEM EQUIVALENT LATERAL FORCE									- THE 10' AN	DIHS UP TO 15' WIDE USE THE "CWO ND 15' WIDTHS ARE SUGGESTED BE A WIDTH IS 15'; (SEE 'ARCHITECTUR,
	PROCEDURE E			STEP 6 LC	AD SCENARI	<u>0</u>					NTIFY SITE SPECIFIC ROOF WIDTH A
	1.25	WHITE			ROOF DEAD LOAD			]	] LOAD SCENARIO 1		EXCEED THE TOTAL ROOF AREA FRO
	Cs x W LOAD SCENARIO = {1,2}	GREEN		TOTA	L ROOF DEAD LOA	D < 2.0 PSF		[	] LOAD SCENARIO 2	STEP 9: FOU	JNDATION TYPE
	Cs = {1.13,1.65} 1.25			STEP 7 P	C STRUCTURE					- SELECT EIT	FOUNDATION BASED THE DESIRED THER SPREAD PAD OR DRILLED PIEF
	1.3			IDTH <= 10 WIDTH <= 15		[ ]CWC				- FOUNDAT	TION TYPE IMPACTS CONSTRUCTIO TION TYPE IMPACTS ANCHOR BOL OF SITE-SPECIFIC SOILS REPORT TO E
ORT PERIOD, Ss - USED TO	LOAD SCENARIO = {1,2} Ss = {1.406, 2.063}		10 11001	Will Control of the C						•	DUNDATION SUMMARY
ADT DEDICO. C.A. LICED TO	1.2				RUCTURE SIZE	<u> </u>		OWO 15		- USE THE SI	elections from Step 6 and Step
PRT PERIOD, Sds - USED TO	LOAD SCENARIO = {1,2} Sds (MAX) = {1.125, 1.650}			CWC 10				CWC 15		STEP 11: SEI	LECT APPLICABLE SHEET INDEX FOR
ECOND PERIOD, \$1	LOAD SCENARIO = {1,2} S1 = {0.844, 1.07}	ROOF WIDTH		[ ] 10'	DEFAULT	[ ] 15'			DEFAULT	- INCLUDE /	THE APPLICABLE SHEET INDEX APPLICABLE SHEETS WITH YOUR DS
	2.0			[ ]OTHER	6' MIN; 10' MAX	[ ]	OTHER	10'-6" M	IN; 15' MAX	- EXCLUDE - EXCLUDE	'MISC DESIGN OPTIONS' SHEET FOR 'ELECTRICAL CUTOUTS' SHEET FOR
COND PERIOD, \$d1	LOAD SCENARIO = {1,2} Sd1 = {1.125, 1.427}			[ ] 44'	2 BAYS	[ ] 36'			2 BAYS	<b>STEP 12</b> : MU	JLTIPLE STRUCTURES WITH THE SAM
HE LIMITS OF THESE PC DRAWINGS	NONE NONE S, CONTACT POLIGON ENGINEERING								0.5.1.10	- UNO ON T	OF LENGTH AND WIDTH OF STRUC THE POLIGON DRAWINGS, POLIGO THE POLIGON DRAWINGS OF STRUCK
AL DRAWINGS, AND/OR CALCUL/ ATION FROM THIS PC MAY NOT BI	ATIONS COULD BE SUBMITTED FOR A	ROOF LENGTH	1	[ ] 64'	3 BAYS	[ ] 52'			3 BAYS		RE IS THE SAME T POLIGON FOR FURTHER INFORMA
PANELS. STRUCTURE IS NOT DESI	GNED TO SUPPORT			[ ] 84'	4 BAYS	[ ] 68'			4 BAYS		DLUMN BASE PROTECTION - SPREA
NS.	ONED TO 3011 ORT			[ ]OTHER		[ ]	OTHER			- SELECT TH NOTE 3 SP	IE METHOD OF COLUMN INSTALLA' PREAD PAD FOUNDATION IS SELEC
A COVERED UNDER THE ROOF EX INE. VERIFY SUBMITTAL AND APPR	(CEEDS 4000 SQ FT OR IS LOCATED POVAL OF A GEOHAZARD REPORT BY			STEP 9 FOI	JNDATION TY	PE					
RUCTURES: CWC10= 2.75" CWC	215= 3.00"	FOUNATION TYPE		CWC 10				CWC 15			
UCTURES: CWC10= 3.75" CWC1	5=4.00"	T GOTWINGTOTTE	[ ] SPRE/	AD PAD [ ] DR	ILLED PIER	[ ] SPR	EAD PAD	[	] DRILLED PIER	· -	WC1.0 ORDER FORM WC1.1 NOTES AND SPECIAL INSPECTI
OOTINGS ADJACENT TO ONE AN HE FOUNDATIONS. THE MINIMUM ACCOMPANYING ENGINEERING	CLEARANCE BETWEEN CENTER OF			STEP 10 FOUN	DATION SUM	MARY					WC2.0 FOUNDATION PLAN SPREAD F WC2.1 FOUNDATION PLAN DRILLED F
SA REVIEWER SHALL VERIFY THE S	STRUCTURE TO BE LOCATED AT LEAST 20		CWC 10				CWC 15			5 C\	WC2.2 FOUNDATION PLAN SPREAD F
F GROUND SNOW LOAD IS GREA		[ ] LOAD SCENA SPREAD PAD	RIO 1	[ ] LOAD SCENARIO 1 DRILLED PIER		D SCENARIO EAD PAD	O 1	[ ] L(	OAD SCENARIO 1 DRILLED PIER		WC2.3 FOUNDATION PLAN DRILLED F WC3.0 FRAMING PLAN - CWC 10
CEPTION 1 OF ASCE 7-16 SECTION ALL EXTEND TO WITHIN 150 FEFT O	OF ALL PORTIONS OF THE PERIMETER										WC3.1 FRAMING PLAN - CWC 15 WC4.0 FRAME CONNECTION DETAILS
	TO THE PERIOD OF THE PERIOD OF	[ ] LOAD SCENA SPREAD PAD	RIO 2	[ ] LOAD SCENARIO 2 DRILLED PIER	[ ] LOA SPR	D SCENARIO EAD PAD	02	[ ] L( [	OAD SCENARIO 2 DRILLED PIER	<del></del>	WC4.1 FRAME CONNECTION DETAILS SHEETS = 16
										101/12	JAMES TO
	DESIGN VALUES  II B				SHEET INDEX						
	1	BASE FRAME ROOF DECK	MR	CWC 10 SHEET INDEX	SS	N	CWC T	15 SHEET IN	SS SS	<u>ABBRI</u>	EVIATIONS:
	NOT BY POLIGON									ACI	AMERICAN CONCRETE INST AMERICAN INSTITUTE OF ST
STANDARDS:		FOUNDATION TYPE	SPREAD PAD	DRILLED SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	AISC ASM	AMERICAN INSTITUTE OF ST CONSTRUCTION ASSEMBLY (INTERNAL REFERE
(Part 1 Title 24 CCP)										ASTM	ASSEMBLY (INTERNAL REFERE  AMERICAN SOCIETY FOR TESTIN  MAT'LS
(Part 1, Title 24, CCR)  nd 2 (Part 2, Title 24, CCR)  lifornia amendments)		SELECT ONE	[ ]		[ ]	[ ]	[ ]	[ ]	[ ]	AWS	AMERICAN WELDING SOC
(Part 3, Title 24, CCR) ornia amendments)		ORDER FORM	CWC1.0	CWC1.0 CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CWC1.0	CBC	CALIFORNIA BUILDING CO
(Part 4, Titlé 24, CCR) lifornia amendments) (Part 5, Title 24, CCR)		NOTES AND SPECIAL INSPECTIONS	CWC1.1	CWC1.1 CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CWC1.1	CJP CLR	COMPLETE JOINT PENETRAT  CLEAR
rnia amendments)(Part 8, Title 24, CCR)		FOUNDATION PLAN FRAMING PLAN	CWC2.0 CWC3.0	CWC2.1 CWC2.0 CWC3.0 CWC3.0	CWC2.1 CWC3.0	CWC2.2	+	CWC2.2 CWC3.1	CWC2.3 CWC3.1	DEG	DEGREE
(Dant 0 Till - 04 000)		FKAMMUULT PLAM	(.77(	₩₩₩₩₩₩₩	C. VV C 5 LJ	<sub>1</sub> ∪ <b>∀</b> ∀ (	CVV(5		C. VV C 5	. 1	

MULTIPLE STRUCTURES

**STEP 12 MULTIPLE STRUCTURES** 

**ROOF WIDTH X LENGTH** 

QTY

	<u> </u>	<u>step 1 project</u>	<u>r inform/</u>	<u>NOITA</u>				SIEF 1. GENERAL PROJECT INFORMATION	PART 1 - GENERAL
T NAME _ DISTRICT								<ul> <li>IDENTIFY PROJECT NAME AND SCHOOL DISTRICT</li> <li>IDENTIFY USE AND OCCUPANCY CLASSIFICATION</li> <li>THE USE AND OCCUPANCY DETERMINE THE MAXIMUM SQUARE FOOTAGE OF THE STRUCTURE</li> </ul>	1.1 STRUCTURE DESCRIPTION  A. STRUCTURE(S) BASED ON THE FOLLOWING PC DESIGN(S):
OCCUPANCY CATION				(PROI	OSED OCC	UPANCY: A	I, A2, A3, A4, A5, B, E	- THE MAXIMUM SQUARE FOOTAGE IS ALSO LIMITED BY THE NUMBER OF OCCUPANTS - IDENTIFY THE OCCUPANT LOAD PER TABLE 1004.5 IN THE CBC	1. WALKWAY COVER (CWC)  1.2 DESIGN REQUIREMENTS
ANT LOAD FACTOR			(15 SQFT/PER				ANY A OCCUPANCY	- IDENTIFY TOTAL ROOF AREA WHICH SHALL NOT EXCEED ALLOWABLE AREA PER TABLE 506.2 IN THE CBC IDENTIFY EXPECTED NUMBER OF OCCUPANTS BASED ON THE ESTIMATED OCCUPANT LOAD - TOTAL ROOF AREA DIVIDED BY OCCUPANT LOAD CAN DETERMINE NUMBER OF OCCUPANTS	A. MEET THE DESIGN INTENT SHOWN ON THE PC DRAWINGS APPROVED FOR THIS PROJECT.  1. DESIGN CRITERIA
OOF AREA				20	3QFI/PERSC	JN MAX FO	R B or E OCCUPANCY	STEP 2: DESIGN OPTIONS	<ul> <li>2. MEMBERS SIZES</li> <li>3. HIDDEN BOLTED CONNECTIONS BETWEEN STRUCTURAL MEMBERS</li> <li>4. COLUMN ANCHORAGE SHALL INCLUDE FOUR (4) BOLTS IN COMPLIANCE WITH OSHA 1926.7</li> </ul>
OF OCCUPANTS								- SELECT ROOF DECK FOR YOUR PROJECT - "MR" REPRESENTS MCELROY METAL "MULTI-RIB" ROOF DECK	5. NO FIELD WELDING PERMITTED 6. NO FIELD PAINTING PERMITTED
		STEP 2 DESIG	GN OPTIO	NS				- "SS" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF DECK - SELECT WHETHER GUTTERS AND DOWNSPOUTS FROM POLIGON IS NEEDED FOR YOUR PROJECT	7. ROOF DIMENSIONS AND SLOPES 8. EXPOSED STEEL ROOF FASTENERS (IF APPLICABLE) POWDER COATED BY MANUFACTURER 9. ROOF DECK SPANS FROM PEAK TO EAVE AND PERMITS PROPER DRAINAGE WITHOUT DEBRIS
ECK		[ ] MULTI-RIB (MR)				DE	FAULT, WEIGHT 1.2 PS	- IF "YES", THEN INCLUDE SHEET CWC7.0 IN THE DRAWING SET -SELECT WHETHER ELECTRICAL CUTOUTS ARE NEEDED FOR YOUR PROJECT	UP.
·		[ ] STANDING SEA	(N) (33)				WEIGHT 1.8 PS <b>DEFAUL</b>	- SHEET CWC7.0 SHOWS ELECTRICAL CUTOUT SIZE AND LOCATION CUTOUTS IN COLUMNS - SHEET CWC7.1 HAS INSTRUCTIONS AND SHEET TO IDENTIFY WHICH COLUMNS - SHEET CWC7.1 MUST BE FILLED OUT IN THE SUBMITTAL SET APPROVED BY DSA	1.3 SUBMITTALS  A. DRAWINGS AND CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE APPROI STATE.
		[ ] YES				SEE	CWC7.0 FOR DETAIL <b>DEFAUL</b>	- IF NOTHING IS FILLED IN ON CWC7.1, POLIGON WILL ASSUME CUTOTUS ARE ONLY IN COLUMN A1 (SEE 'FRAMING PLAN' FOR REFERENCE)	B. ONLY MANUFACTURERS THAT SUBMIT DRAWINGS AND CALCULATIONS PRIOR TO BID SHALL BE CONSIDERED.
CAL ACCESS		[ ] YES				SEE	CWC7.1 FOR DETAIL	- SELECT CLEAR HEIGHT (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE) - MIN 7'-1"; MAX 10'-0" - IF NOTHING IS SELECTED, POLIGON WILL ASSUME THE DEFAULT FOR EACH DESIGN OPTION	C. MANUFACTURER MUST BE ABLE TO SUBMIT APPROPRIATE LABORATORY TESTS FOR THE FOLLOWING  1. FRAME FINISH REQUIREMENTS LISTED IN PART 2 OF THIS SPECIFICATION.
IEIGHT		[ ] 8' [ ] OTHER					DEFAUL 10' MA		<ol> <li>CERTIFIED MILL TEST REPORTS FOR STRUCTURAL STEEL (DESCRIBING THE CHEMICAL AND PHYS PROPERTIES).</li> <li>CERTIFIED MILL TEST REPORTS FOR STRUCTURAL BOLTS.</li> </ol>
		STEP 3 SEISMIC	ACCELER/	ΔΤΙΟΝ				STEP 3: IDENTIFY THE Ss & S1 ACCELERATION (g) FOR YOUR PROJECT AND GEOTECHNICAL INFORMATION - Ss & S1 VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES	1.4 TECHNICAL SUPPORT
		·		(g)				<ul> <li>- Ss &amp; S1 VALUE DEPENDS ON PROJECT'S GEOGRAPHICAL LOCATION</li> <li>- FIND Ss &amp; S1 VALUES FOR YOUR PROJECT IN THE SITE SPECIFIC GEOTECHNICAL REPORT</li> </ul>	A. MANUFACTURER MUST HAVE IN-HOUSE ENGINEERING DEPARTMENT AND A PROFESSIONAL ENGIN LICENSED IN THE APPROPRIATE STATE TO ANSWER TECHNICAL QUESTIONS.
	\$1	·		(g)				<ul> <li>FIND Ss &amp; S1 VALUES FOR YOUR PROJECT USING <a href="https://asce7hazardtool.online/">https://asce7hazardtool.online/</a></li> <li>THIS PC IS NOT APPROVED FOR Ss VALUES GREATER THAN 2.063 (CONTACT POLIGON FOR</li> </ul>	1.5 QUALITY ASSURANCE A. GENERAL
		STED A SEISAA	NC DECIO	NC				ADDITIONAL OPTIONS)  STEP 4: IDENTIFY THE SEISMIC REGION FOR YOUR PROJECT	<ol> <li>FABRICATION PROCEDURES SHALL COMPLY WITH APPLICABLE CODES AND LOCAL REGULATION.</li> <li>REQUIRED STRUCTURAL TESTS AND SPECIAL INSPECTIONS INCLUDED ON THE PROJECT DSA-1.</li> <li>B. MANUFACTURER QUALIFICATIONS</li> </ol>
s <= 1.406	x=0.844	STEP 4 SEISM	IIC REGIO	<u>IN3</u>		3.5	PSF MAX DEAD LOAI	- THE REGIONS ARE DEPENDANT ON THE Ss & S1 VALUES DETERMINED IN STEP 3	1. MINIMUM (10) YEARS ENGINEERING AND EABRICATING PRE-ENGINEERED STRUCTURES
Ss <= 2.063 S1 <	z=1.070	[ ] GREEN				2.0	PSF MAX DEAD LOAI	- THE SEISMIC REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED (SEE TABLE TO THE LEFT)  STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT	<ol> <li>MANUFACTURER OWNED AND OPERATED POWDER COAT PAINT FINISH SYSTEM</li> <li>ALL AWS CERTIFIED WELDERS</li> <li>FULL-TIME PROFESSIONAL ENGINEER ON STAFF LICENSED IN THE APPROPRIATE STATE</li> </ol>
	S	STEP 5 TOTAL RC	OOF DEAD	LOAD				- THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED	5. FULL-TIME AWS CERTIFIED ASSOCIATE WELDING INSPECTOR ON STAFF 6. FULL-TIME QUALITY ASSURANCE MANAGER ON STAFF 7. FULL-TIME LEED AP ON STAFF
								- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME - TOTAL ROOF DEAD LOAD MUST BE LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4	C. MANUFACTURER CERTIFICATIONS  1. PCI 4000 CERTIFICATION THROUGH POWDER COATING INSTITUTE (PCI)
ECK		PSF			SEE S	STEP 2' 'ROC	F DECK' FOR WEIGHT	- CUT SHEETS OF ANY BOARDS, BOXES AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE, INCLUDING WEIGHTS AND DIMENSIONS ARE REQUIRED	2. AISC CERTIFIED FABRICATOR  1.6 MANUFACTURER WARRANTY
ERAL		. PSF	:			ICHTING F	RE SUPPRESSION, ETC	STEP 6: IDENTIFY THE LOAD SCENARIO	A. STRUCTURE MUST HAVE (10) YEAR LIMITED WARRANTY ON STEEL FRAME MEMBERS. B. STRUCTURE MUST HAVE (10) YEAR LIMITED WARRANTY ON PAINT SYSTEM.
LIVAL		131			L	IGITIING , I	RE 3011 RE331OIN, ETC	- REFERENCE THE STEP 4 COLOR AND SELECT THE APPLICABLE LOAD SCENARIO - LOAD SCENARIOS HAVE NO IMPACT ON FRAME DESIGN OR COST, BUT DO AFFECT FOUNDATION SIZE	C. PASS THROUGH WARRANTY OF ROOFING MANUFACTURER SHALL BE PROVIDED UPON REQUEST.  PART 2 - PRODUCTS
		PSF	:		ADE	) 'ROOF DEC	CK' AND 'COLLATERA	STEP 7: IDENTIFY PC STRUCTURE	2.1 MANUFACTURER A. ACCEPTABLE MANUFACTURERS
								- ROOF WIDTHS UP TO 10' WIDE USE THE "CWC 10" - ROOF WIDTHS UP TO 15' WIDE USE THE "CWC 15" - THE 10' AND 15' WIDTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST ECONOMICAL	1. POLIGON, A DIVISION OF PORTERCORP. A. 4240 N 136TH AVE., HOLLAND, MI 49424; (616) 399-1963; WWW.POLIGON.COM .
		STEP 6 LOAI	D SCENAR	10				- MAXIMUM WIDTH IS 15'; (SEE 'ARCHITECTURAL VIEWS' SHEET FOR REFERENCE)	I. FOR POLIGON STRUCTURES IN <i>NORTHERN CALIFORNIA,</i> THE LOCAL REPRESENTATI ABOUT PLAY( <u>WWW.PLAYGROUNDPROS.COM</u> ). EMAIL <u>AAP@PLAYGRAOUNDPRO</u> OR CALL (916) 923-2180
	,		OF DEAD LOA				LOAD SCENARIO	STEP 8: IDENTIFY SITE SPECIFIC ROOF WIDTH AND LENGTH	II. FOR POLIGON STRUCTURES IN SOUTHERN CALIFORNIA, THE LOCAL REPRESENTAT MIRACLE PLAYGROUND SALES (WWW.MIRACLEPLAYGROUNDSALES.COM). EMA
		TOTAL RC	OOF DEAD LO	AD < 2.0 PSF		[	] LOAD SCENARIO	- DO NOT EXCEED THE TOTAL ROOF AREA FROM STEP 1 (ROOF WIDTH MULTIPLIED BY ROOF LENGTH)  STEP 9: FOUNDATION TYPE	SALES@MIRACLEPLAYGROUND.COM OR CALL (951) 695-4515  B. SUBSTITUTION LIMITATIONS  1. THE ENCINEEDING FOR THIS STRUCTURE IS ONLY APPLICABLE IF ROUGON SUPPLIES THE MATERIAL PROPERTY.
		STEP 7 PC	STRUCTURI	F	,			- SELECT A FOUNDATION BASED THE DESIRED FOUNDATION TYPE	1. THE ENGINEERING FOR THIS STRUCTURE IS ONLY APPLICABLE IF POLIGON SUPPLIES THE MATE 2. IF THE CONTRACTOR ELECTS TO SUBSTITUTE A DIFFERENT STRUCTURE, THEY ARE RESPONSIBLE TO OBTAIN THE NECESSARY DSA APPROVAL WITH:
	ROOF WIDTH <= 1		<u> </u>	[ ] CWC	10			<ul> <li>SELECT EITHER SPREAD PAD OR DRILLED PIER FOUNDATION PRIOR TO APPROVAL</li> <li>FOUNDATION TYPE IMPACTS CONSTRUCTION (TIMING, SEQUENCE, COST, ETC.)</li> <li>FOUNDATION TYPE IMPACTS ANCHOR BOLT LENGTH (NOT PROVIDED BY POLIGON)</li> </ul>	A. NO COST TO THE DISTRICT OR ARCHITECT  B. NO CHANGE TO THE CONSTRUCTION SCHEDULE  3. SUBSTITUTIONS AND APPROVED A MINIMUM AND ANS REFORE BLD.
	10 < ROOF WIDTH <	= 15		[ ] CWC	15			- REVIEW OF SITE-SPECIFIC SOILS REPORT TO EVALUATE APPLICABILITY OF FOUNDATION OPTIONS AVAILABLE	3. SUBSTITUTIONS MUST BE APPROVED A MINIMUM OF (10) DAYS BEFORE BID. 4. ALL APPROVED MANUFACTURERS SHALL BE NOTIFIED IN WRITING BEFORE THE BID DATE. 5. SUBSTITUTE MANUFACTURERS SHALL NOT BE ALLOWED TO BID WITHOUT WRITTEN
		STEP 8 STRU	JCTURE SIZ	<u>′E</u>				STEP 10: FOUNDATION SUMMARY - USE THE SELECTIONS FROM STEP 6 AND STEP 9 TO SELECT THE APPROPRIATE FOUNDATION	NOTIFICATION. 6. SUBSTITUTE MANUFACTURERS MUST MEET "MANUFACTURER QUALIFICATIONS" LISTED IN
		CWC 10				CWC 15		STEP 11: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT	PART 1 OF THIS SPECIFICATION. 7. SUBSTITUTE MANUFACTURERS MUST PROVIDE PROOF OF "MANUFACTURER CERTIFICATIONS" ABOVE.
ROOF WIDTH	[ ] 10'	DE	EFAULT	[ ] 15'		ı	DEFAULT	- IDENTIFY THE APPLICABLE SHEET INDEX - INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL	8. SUBSTITUTE MANUFACTURERS MUST PROVIDE PAINT FINISH DESCRIBED IN "FRAME FINISH" BELOW.
KOOT WIDTH	[ ]	OTHER 6' N	MIN; 10' MAX	[ ]	OTHER	10'-6" N	IN; 15' MAX	- EXCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS - EXCLUDE 'ELECTRICAL CUTOUTS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS	2.2 FRAME A. MATERIALS
	[ ] 44'	2	2 BAYS	[ ] 36'			2 BAYS	STEP 12: MULTIPLE STRUCTURES WITH THE SAME PC#	<ol> <li>ANCHOR BOLTS: SEE DRAWINGS FOR REQUIREMENTS. ANCHOR BOLTS NOT PROVIDED BY MANUFACTURER.</li> </ol>
	[ ]		- 5, 110	[ ] 00			2 57 (10	- FILL IN ROOF LENGTH AND WIDTH OF STRUCTURES AS WELL AS QUANTITY - UNO ON THE POLIGON DRAWINGS, POLIGON WILL ASSUME ALL DESIGN CRITERIA FOR EACH	2. STRUCTURAL STEEL: SEE DRAWINGS FOR REQUIREMENTS. 3. STRUCTURAL BOLTS: SEE DRAWINGS FOR REQUIREMENTS. B. FINISH
ROOF LENGTH	[ ] 64'		3 BAYS	[ ] 52'			3 BAYS	STRUCTURE IS THE SAME - CONTACT POLIGON FOR FURTHER INFORMATION	<ol> <li>FRAME FINISH: POLI-5000 POWDER COAT. NO FIELD PAINTING PERMITTED.</li> <li>COMPONENTS SHALL BE CLEANED, PRE-TREATED, AND FINISHED AT A FACILITY OWNED</li> </ol>
	[ ] 84'	4	4 BAYS	[ ] 68'			4 BAYS	STEP 13: COLUMN BASE PROTECTION - SPREAD PAD FOUNDATION SELECTION ONLY	DIRECTLY SUPERVISED BY THE MANUFACTURER.  B. COMPONENTS SHALL BE SHOT BLASTED TO SSPC-SP10 NEAR-WHITE BLAST CLEANING.
	[ ]	OTHER		[ ]	OTHER			- SELECT THE METHOD OF COLUMN INSTALLATION ON APPLICABLE FOUNDATION PLAN SHEET, DETAIL 2, NOTE 3 SPREAD PAD FOUNDATION IS SELECTED	HAND TOOL CLEANING WILL NOT BE AN ACCEPTABLE ALTERNATIVE. C. COMPONENTS SHALL BE PRETREATED IN A (3) STAGE IRON PHOSPHATE OR EQUAL WAD. COMPONENTS SHALL RECEIVE EPOXY PRIMER COAT FOR SUPERIOR CORROSION PRO
		STEP 9 FOUN	DATION TY	YPE					<ul> <li>E. COMPONENTS SHALL RECEIVE TOP COAT OF SUPER DURABLE TGIC POWDER COAT.</li> <li>F. FINISH SHALL NOT HAVE ANY VOC EMISSIONS.</li> <li>G. MANUFACTURER SHALL BE ABLE TO PRODUCE DOCUMENTATION STATING SAMPLE</li> </ul>
UNATION TYPE		CWC 10				CWC 15		SHEET INDEX	PRODUCTION COMPONENTS HAVE BEEN TESTED TO MEET THE FOLLOWING:  1. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654 TO 10,000 HOURS WITH NO
	[ ] SPREAD PAD	[ ] DRILLE	ED PIER	[ ] SPR	EAD PAD	] [	] DRILLED PIER	1 CWC1.0 ORDER FORM 11 CWC5.0 ARCHITECTURAL VIEWS - CWC 10 2 CWC1.1 NOTES AND SPECIAL INSPECTIONS 12 CWC5.1 ARCHITECTURAL VIEWS - CWC 15	FROM SCRIBE LINE AND RATING OF 10. II. HUMIDITY RESISTANCE PER ASTM D2247-02 TO 5,000 HOURS WITH NO LOSS OF AE
	<u>S</u>	TEP 10 FOUNDA	ATION SUM	MARY				3 CWC2.0 FOUNDATION PLAN SPREAD PAD - CWC 10 13 CWC6.0 ROOF CONNECTION DETAILS 4 CWC2.1 FOUNDATION PLAN DRILLED PIER - CWC 10 14 CWC6.1 ROOF CONNECTION DETAILS	OR BLISTERING. III. COLOR/UV RESISTANCE PER ASTM G154-04 TO 2,000 HOURS EXPOSURE, ALTERN, CYCLES WITH RESULTS OF NO CHALKING, 75% COLOR RETENTION, COLOR VARIA
	CWC 10				CWC 15	<u> </u>		5 CWC2.2 FOUNDATION PLAN SPREAD PAD - CWC 15 15 CWC7.0 MISC DESIGN OPTIONS 6 CWC2.3 FOUNDATION PLAN DRILLED PIER - CWC 15 16 CWC7.1 ELECTRICAL CUTOUTS	maximum 3.0 e Variation cie formula (before and after 2,000 hours exf 2. frame color: determined by district.
[ ] LOAD SCENAR SPREAD PAD	RIO 1 [ ] LO.	AD SCENARIO 1 RILLED PIER	[ ] LO/ SPF	AD SCENARIO READ PAD	) l	[ ] L	DAD SCENARIO 1 DRILLED PIER	7 CWC3.0 FRAMING PLAN - CWC 10	C. FABRICATION  1. FABRICATE COMPONENTS TO PERMIT BOLTED CONNECTIONS ON SITE. NO FIELD WELDING PERMITTED.
		AD COENTABIO O	[ ]]	A D COENTA DI	2.0	f 11.		8 CWC3.1 FRAMING PLAN - CWC 15 9 CWC4.0 FRAME CONNECTION DETAILS - CWC 10	2. LABEL EACH MEMBER WITH UNIQUE PART NUMBER TO STREAMLINE ERECTION. 3. WELDING REQUIREMENTS: SEE DRAWINGS FOR REQUIREMENTS.
[ ] LOAD SCENAR SPREAD PAD	NO 2 [ ] LO.	AD SCENARIO 2 RILLED PIER	l J.LO/ SPI	AD SCENARIO READ PAD	J		OAD SCENARIO 2 DRILLED PIER	10 CWC4.1 FRAME CONNECTION DETAILS - CWC 15 TOTAL SHEETS = 16	2.3 ROOF A. MATERIALS
									<ol> <li>ROOF MATERIAL: SEE DRAWINGS FOR REQUIREMENTS.</li> <li>ROOF HARDWARE: SEE DRAWINGS FOR REQUIREMENTS.</li> </ol>
		STEP 11 SH	HEET INDEX	<u> </u>	CWC	. 1. CHEET IN	IDEV		B. FINISH  1. ROOF FINISH: KYNAR 500 HIGH-PERFORMANCE RESIN-BASED PAINT.
BASE FRAME ROOF DECK	MR	C 10 SHEET INDEX		٨	AR CWC	15 SHEET IN	SS	ABBREVIATIONS:	2. ROOF COLOR: DETERMINED BY OWNER. <b>2.4 MISCELLANEOUS</b>
								ACI AMERICAN CONCRETE INSTITUTE MR MULTI-RIB ROOF PANEL (MCELROY)  AUSC AMERICAN INSTITUTE OF STEEL DITS NOT TO SCALE	A. MATERIALS  1. CONCRETE MATERIAL: SEE DRAWINGS FOR REQUIREMENTS. CONCRETE NOT PROVIDED BY
JNDATION TYPE	SPREAD PAD DRILLED PIER	SPREAD PAD E	ORILLED PIER	SPREAD PAD	DRILLED PIER	SPREAD PAD	DRILLED PIER	AISC AMERICAN INSTITUTE OF STEEL NTS NOT TO SCALE  ASM ASSEMBLY (INTERNAL REFERENCE) NO NUMBER	MANUFACTURER. PART 3 - EXECUTION
								ASTM AMERICAN SOCIETY FOR TESTING AND OC ON CENTER	3.1 STORAGE AND HANDLING
SELECT ONE	[ ] [ ]	[ ]	[ ]	[ ]	[ ]	[ ]	[ ]	AWS AMERICAN WELDING SOCIETY OSHA OCCUPATIONAL HEALTH AND SAFETY ADM.	A. PROTECT MATERIAL AFTER DELIVERY FROM WEATHER, SUNLIGHT, AND DAMAGE.  B. ELEVATE MATERIAL TO ALLOW CIRCULATION AND REDUCE MOLD, FUNGI DECAY, AND INSECT INFORMATION AND REDUCE MOLD, FUNGI DECAY, FUNGI PROPERTING FUNGI PROPERTING FUNGI PROPERTING FUNGI PROPERTING FUNGI PROPERTING FUNGI PROPERTING FU

ACI	AMERICAN CONCRETE INSTITUTE	IVIK	MULTI-RIB ROOF PANEL (MICELROT)	Λ.
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	NTS	NOT TO SCALE	
ASM	ASSEMBLY (INTERNAL REFERENCE)	NO	NUMBER	PART 3 -
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	ОС	ON CENTER	3.1 STOR
AWS	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.	A. B.
СВС	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT	C.
CJP	COMPLETE JOINT PENETRATION	PD	POLIGON DRAWING	D.
CLR	CLEAR	PJ	PRETENSIONED JOINT	
DEG	DEGREE	PLCS	PLACES	<b>3.2 EREC</b> A.
DIA	DIAMETER	PLT	PLATE	В.
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT	C.
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH	D.
EQ	EQUAL	QTY	QUANTITY	D.
FT	FFFT	RFF	REFERENCE	F

•	, 100 E. 1. 12 E. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	•		' <i>-</i>
STM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	ОС	ON CENTER	3.1
WS	AMERICAN WELDING SOCIETY	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADM.	
ВС	CALIFORNIA BUILDING CODE	PCF	POUNDS PER CUBIC FOOT	
JP	COMPLETE JOINT PENETRATION	PD	POLIGON DRAWING	
CLR	CLEAR	PJ	PRETENSIONED JOINT	] ,
EG	DEGREE	PLCS	PLACES	3.2
NΑ	DIAMETER	PLT	PLATE	
MI	DIMENSION	PSF	POUNDS PER SQUARE FOOT	]
SA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH	
Q	EQUAL	QTY	QUANTITY	
FT	FEET	REF	REFERENCE	
ЭΑ	GAGE	SQ	SQUARE	
Ν	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)	
(SI	KIPS PER SQUARE INCH	TYP	TYPICAL	3.3
АХ	MAXIMUM	UNO	unless noted otherwise	
۸IN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY	3.4
ISC	MISCELLANEOUS	W/	WITH	
				1

MILES PER HOUR

**SPECIFICATIONS** 

PART 1 - GENERAL 1.1 STRUCTURE DESCRIPTION

4. COLUMN ANCHORAGE SHALL INCLUDE FOUR (4) BOLTS IN COMPLIANCE WITH OSHA 1926.755(A)(1). 5. NO FIELD WELDING PERMITTED 6. NO FIELD PAINTING PERMITTED 7. ROOF DIMENSIONS AND SLOPES

8. EXPOSED STEEL ROOF FASTENERS (IF APPLICABLE) POWDER COATED BY MANUFACTURER 9. ROOF DECK SPANS FROM PEAK TÒ EAVE AND PÉRMITS PROPER DRAINAGE WITHOUT DEBRIS BUILD-

A. DRAWINGS AND CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE APPROPRIATE

1. FABRICATION PROCEDURES SHALL COMPLY WITH APPLICABLE CODES AND LOCAL REGULATIONS. 2. REQUIRED STRUCTURAL TESTS AND SPECIAL INSPECTIONS INCLUDED ON THE PROJECT DSA-103 FORM. B. MANUFACTURER QUALIFICATIONS

1. POLIGON, A DIVISION OF PORTERCORP. A. 4240 N 136TH AVE., HOLLAND, MI 49424; (616) 399-1963; <u>WWW.POLIGON.COM</u>.
I. FOR POLIGON STRUCTURES IN NORTHERN CALIFORNIA, THE LOCAL REPRESENTATIVE IS ALL ABOUT PLAY(WWW.PLAYGROUNDPROS.COM). EMAIL AAP@PLAYGRAOUNDPROS.COM

1. FRAME FINISH: POLI-5000 POWDER COAT. NO FIELD PAINTING PERMITTED. A. COMPONENTS SHALL BE CLEANED, PRE-TREATED, AND FINISHED AT A FACILITY OWNED AND DIRECTLY SUPERVISED BY THE MANUFACTURER.

B. COMPONENTS SHALL BE SHOT BLASTED TO SSPC-SP10 NEAR-WHITE BLAST CLEANING. SSPC-SP2 HAND TOOL CLEANING WILL NOT BE AN ACCEPTABLE ALTERNATIVE. C. COMPONENTS SHALL BE PRETREATED IN A (3) STAGE IRON PHOSPHATE OR EQUAL WASHER. D. COMPONENTS SHALL RECEIVE EPOXY PRIMER COAT FOR SUPERIOR CORROSION PROTECTION. COMPONENTS SHALL RECEIVE TOP COAT OF SUPER DURABLE TGIC POWDER COAT.

. FINISH SHALL NOT HAVE ANY VOC EMISSIONS. G. MANUFACTURER SHALL BE ABLE TO PRODUCE DOCUMENTATION STATING SAMPLE PRODUCTION COMPONENTS HAVE BEEN TESTED TO MEET THE FOLLOWING: I. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654 TO 10,000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10.

II. HUMIDITY RESISTANCE PER ASTM D2247-02 TO 5,000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING. III. COLOR/UV RESISTANCE PER ASTM G154-04 TO 2,000 HOURS EXPOSURE, ALTERNATE CYCLES WITH RESULTS OF NO CHALKING, 75% COLOR RETENTION, COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2,000 HOURS EXPOSURE).

SPECIFICATIONS.

REQUIRED BY OTHERS.

A. PROTECT MATERIAL AFTER DELIVERY FROM WEATHER, SUNLIGHT, AND DAMAGE.

B. ELEVATE MATERIAL TO ALLOW CIRCULATION AND REDUCE MOLD, FUNGI DECAY, AND INSECT INFESTATION. HANDLE MATERIAL WITH PROTECTIVE STRAPS OR PADDED FORKLIFT. HANDLING MATERIAL WITH CHAIN OR

BEFORE INSTALLATION AND COVER IMMEDIATELY WITH ANY SECONDARY ROOF MATERIAL.

. INSTALL COMPONENTS ACCORDING TO MANUFACTURER'S INSTALLATION DRAWINGS AND THESE

TO PREVENT MOISTURE DAMAGE TO ANY WOOD MATERIAL (IF APPLICABLE), KEEP WOOD PACKAGED

ANCHOR BOLT AND COLUMN LAYOUT IS CRITICAL COMPLY WITH SPECIFIC BOLTING INSTALLATION REQUIREMENTS PROVIDED ON DRAWINGS REQUIRING THE

CABLE WILL NOT BE ACCEPTED AND MAY VOID MANUFACTURER'S WARRANTY.

CONTRACTOR TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. NO FIELD SLOTTING OR OPENING OF HOLES WILL BE ALLOWED. TOLERANCES ON STEEL STRUCTURAL

MEMBERS ARE SET ACCORDING TO AISC CONSTRUCTION PRACTICES, FOLLOWED DURING FABRICATION, AND CANNOT BE INCREASED. E. AFTER INSTALLATION, RESTORE DAMAGED SURFACES TO THE ORIGINAL CONDITION USING TOUCH-UP PAINT PROVIDED BY MANUFACTURER. IF THE ARCHITECT DOES NOT ACCEPT THAT, REPLACE DAMAGED

MATERIAL AT NO COST TO THE DISTRICT. F. COORDINATE AS REQUIRED WITH OTHER DISCIPLINES (ELECTRICAL, PLUMBING, ETC.) G. COMPLY WITH ALL APPLICABLE OHSA REQUIREMENTS.

A. TESTS AND INSPECTIONS DURING ERECTION ARE NOT REQUIRED BY THE MANUFACTURER, BUT MAY BE

B. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH

DSA PR 13-01. DO NOT PROCEED UNTIL UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED

A. DO NOT ATTEMPT ANY FIELD CHANGES TO THE STRUCTURE WITHOUT FIRST CONTACTING THE MANUFACTURER. .4 QUALITY CONTROL

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STATE APPROVALS-SITE

STATE APPROVALS-PC

IDENTIFICATION STAMP

DIV. OF THE STATE ARCHITECT

REVIEWED FOR

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APP: 02-121211 PC

DATE: 7/14/2023

#### **GENERAL**:

- GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
- WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE
- ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED, TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
- SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE
- REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT'S INSPECTOR OF RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIRMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
- 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.
- 14. OTHER SITE SPECIFIC ITEMS MAY BE REQUIRED.
- 15. WHEN A SITE-SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE

#### STRUCTURAL AND MISCELLANEOUS STEEL:

- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360-16 AND 303-16 REFERENCED BY THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE.
- PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 ksi, GRADE B UNLESS NOTED OTHERWISE.
- STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A500, GRADE B (OR HIGHER), Fy = 46 KSI.
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESSES CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- ALL CHANNELS, ANGLES, PLATES AND MISC. STEEL SHALL CONFORM TO ASTM A36, Fy = 36 KSI.
- 6. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A653, CS = TYPE B, Fy = 50 KSI.
- 7. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1
- ROOF DECK SHALL HAVE KYNAR 5000 METAL COATING.
- ROOF DECK SHALL CONFORM TO ATSM A792, Fy = 50 KSI.
- 10. MR ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.31" (FLAT-TO-FLAT) AND INTEGRAL WASHER DIMENSION OF 0.58" (OUTSIDE DIAMETER).
- 11. SS ROOF SCREWS MEET ASTM A510 WITH A HEAD DIMENSION OF 0.437" (OUTSIDE DIAMETER).

- ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED.
- ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb @ (O° F).
- ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
- WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

#### **BOLTING:**

ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 (A325 TYPE 1) HIGH STRENGTH BOLTS (UNO) AND SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329.

- HIGH STRENGTH BOLTS SHALL BE SAMPLED AND TESTED IN COMPLANCE WITH CBC 2213A.1.
- BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
- ANCHOR BOLTS (HEAVY HEX HEAD, ASTM F1554, GRADE 55) SHALL BE HOT DIPPED GALVANIZED PER ASTM F2329. ANCHOR BOLTS MAY BE HEADED OR THREADED WITH A NUT THAT IS PREVENTED FROM ROTATING.
- HIGH STRENGTH NUTS SHALL CONFORM TO ASTM A563 AND SHALL BE GALVANIZED PER ASTM F2329.
- HIGH STRENGTH WASHERS SHALL CONFORM TO ASTM F436 AND SHALL BE GALVANIZED PER ASTM F2329.
- THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD <u>PRIOR TO THE ERECTION OF THE FRAME</u>. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
- A. PRETENSIONED JOINTS (IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH A "PJ REQUIRED") MUST BE INSTALLED AND INSPECTED TO MEET ONE OF FOLLOWING REQUIREMENTS:
  - 1. TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRENTENSIONING
  - 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF REQUIRED WASHERS)
- B. ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF SNUG-TIGHTENED JOINTS. NOTE TO INSTALLER AND INSPECTOR(S): THE SNUG-TIGHT CONDITION EXISTS, IN PART, WHEN ALL THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

THE CONTRACTOR, SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD MUST ALL AGREE ON WHICH APPROACH WILL BE USED TO PRETENSION THE BOLTS. THE CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING THE APPROACH AGREED TO BY ALL PARTIES LISTED ABOVE.

#### **FOUNDATIONS:**

- ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER 2022 CBC TABLE 1806A.2
- FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D1557. FLOODING NOT PERMITTED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECCESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- STRUCTURES SHALL BE SETBACK FROM ADAJCENT SLOPES TO PROVIDE FIRM MATERIAL FOR EMBEDMENT AND FOR PROTECTION FROM SLOPE DRAINAGE, EROSION, AND SHALLOW FAILURES.
  - A. BOTTOM OF ASCENDING SLOPE: THE SMALLER OF HALF THE HEIGHT OF THE SLOPE AND 15FT MEASURED FROM THE FACE OF THE STRUCTURE TO THE TOE OF THE SLOPE B. TOP OF DECENDING SLOPE: THE SMALLER OF A THIRD OF THE HEIGHT OF THE SLOPE AND 40 FT MEASURED
- FROM THE FACE OF THE FOOTING TO THE TOP OF THE SLOPE ALTERNATE SETBACKS ARE PERMITTED, SUBJECT FOR APPROVAL. A GEOTECHNICAL INVESTIGATION MAY BE
- STRUCTURES PLACED ON LIQUIFIABLE SOILS OR SITE CLASS F MAY NOT BE SUBMITTED FOR AN OVER THE COUNTER REVIEW.

MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

MINIMUM STRENGTH f'c	EXPOSURE CATEGORY	MAXIMUM	SLUMP	UNIT WEIGHT
(28 DAYS)		W/C RATIO	(± 1'')	(NORMAL WEIGHT)
5000 PSI	F3, S3, W2, C2	0.4	4"	150 PCF

- CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA
- AGGREGATES SHALL CONFORM TO ASTM C33. MAX AGGREGATE SIZE = 1".
- CEMENT SHALL CONFORM TO ASTM C150 (TYPE V) WITH A MAXIMUM EXPANSION OF 0.040%, FOR SULFATE RESISTANCE.
- ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE PROHIBITED.
- CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318-19 SECTION 19.3.3.
- CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
- CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- CONCRETE SHALL BE PROPORTIONED PER ACI 318-19 26.4.
- CONCRETE SHALL BE TESTED PER CBC 1910A.1, 1705A.3, AND ACI 318-19 26.13. BATCH PLANT INSPECTION NOT REQUIRED. CONTRACTOR SHALL IMPLEMENT WEIGHTMASTER AND BATCH TICKET REQUIREMENTS OF CBC 1705A.3.3.1.

- REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A615, (DEFORMATIONS SHALL BE IN ACCORDANCE WITH ASTM A305) AS FOLLOWS: GR 60: (#4 BARS AND LARGER)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
  - A. CAST AGAINST EARTH. CAST AGAINST FORM BELOW GRADE FORMED SLABS (#11 BAR & SMALLER) SLABS ON GRADE (FROM TOP OF SLAB). COLUMNS AND BEAMS (MAIN BARS). F. WALLS EXPOSED TO WEATHER (#6-#18 BARS). (#5 & SMALLER)
- G. NOT EXPOSED TO WEATHER (#11 & SMALLER)...... 3/4" BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
- FOR #6 BARS AND SMALLER, REINFORCING SHALL BE LAP SPLICED 45 BAR DIA MINIMUM IN CONCRETE. FOR #7 BARS AND LARGER, REINFORCING SHALL BE LAP SPLICED 55 BAR DIAMETERS MINIMUM IN CONCRETE. ALL LAP SPLICES MUST COMPLY WITH
- PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- WELDING OF REINFORCING IS NOT ALLOWED REINFORCING STEEL SHALL BE SAMPLED AND TESTED PER CBC 1910A.2.

#### **POWDER COATED AND EPOXY PRIMED FINISH:**

- ENTIRE POWDER COATING PROCESS COMPLETED IN SAME FACILITY AS STEEL FABRICATION.
- ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED OTHERWISE).
  - PARTS PRETREATED IN A 3 STAGE IRON PHOSPHATE WASHER (OR EQUAL).
- EPOXY PRIMER POWDER COAT APPLIED TO PARTS FOR SUPERIOR CORROSION PROTECTION.
- TOP POWDER COAT OF SUPER DURABLE TGIC (COLOR SELECTED FROM MANUFACTURER'S STANDARD OPTIONS OR CUSTOM COLOR).
- SAMPLE PRODUCTION PARTS TESTED TO MEET THE FOLLOWING CRITERIA: A. SALT SPRAY RESISTANCE PER ASTM B 117/ ASTM D 1654
  - 1. 10000 HOURS WITH NO CREEP FROM SCRIBE LINE AND RATING OF 10
  - B. HUMIDITY RESISTANCE PER ASTM D2247-02 1. 5000 HOURS WITH NO LOSS OF ADHESION OR BLISTERING C. COLOR/UV RESISTANCE PER ASTM G154-04
  - 1. 2000 HOURS EXPOSURE ALTERNATE CYCLES WITH NO CHALKING, 75% COLOR RETENTION, AND COLOR VARIATION MAXIMUM 3.0 E VARIATION CIE FORMULA (BEFORE AND AFTER 2000 HOURS

#### **CONSTRUCTION NOTES**

- . A DSA-CERTIFIED CLASS 2 INSPECTOR IS REQUIRED FOR THIS PROJECT.
- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQURIED BY SECTION 4-338, PART 1, TITLE 24 CCR, AND DSA IR A-6.
- A "DSA-CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE OWNER (E.G. DISTRICT, ETC.) SHALL CONDUCT ALL THE REQUIRED TEST AND INSPECTIONS FOR THE PROJECT.

#### NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEER RESPONIBILITY

- FOR THE SITE-SPECIFIC PROJECT, NEITHER POLIGON OR GHD ARE THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
- FOR THE SITE-SPECIFIC PROJECT, GHD AND POLIGON'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC ONLY.
- STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GHD AND POLIGON'S RESPONSIBILITY FOR THE SITE-SPECIFIC PROJECT.
- ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING MAY BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATIONS OF CONSTRUCTION, REVIEW OF INSPECTIONS REPORTS, AND SIGNING OFF ON THE VERIFIED REPORT FOR COMPLETED WORK.
- POLIGON WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE STRUCTURES OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

#### **SPECIAL INSPECTION NOTES:**

S1. GENERAL:

✓ d. Test concrete (f'₀)

e. Batch plan inspection:

S/A2. HIGH-STRENGTH BOLTS:

S/A3. WELDING:

Test or Special Inspection

Test or Special Inspection

S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):

- 1. THE PROJECT INSPECTOR AND TESTING AGENCY SHALL BE SELECTED BY THE SCHOOL DISTRICT AND APPROVED BY DSA AND THE ARCHITECT OF RECORD.
- 2. COSTS OF THE PROJECT INSPECTOR AND THE TESTING AGENCY SHALL BE BORN BY THE SCHOOL DISTRICT.
- 3. THE PROJECT INSPECTOR, AND ENTIRE CONSTRUCTION OVERSIGHT PROCESS, SHALL COMPLY WITH DSA PR 13-01.
- ON APPROVED PC DRAWINGS, THE STATEMENT OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS (FORM DSA-103) BELOW<u>IS ONLY AN EXAMPLE</u>. ON APPROVED PC DRAWINGS, THE EXAMPLE FORM DSA-103 MUST BE CROSSED OUT BEFORE THE PC DRAWINGS CAN BE APPROVED AS PART OF A SITE-SPECIFIC (OR STOCKPILE) PROJECT SO THEY WILL NOT CONFLICT WITH THE OFFICIAL FORM DSA-103 FOR THE PROJECT.

#### DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC

Application Number:	School Name:	School District:
DSA File Number:	Increment Number:	Date Submitted:
	2022 CBC	
Generally, the structural tests and special inspections Inspector. The actual complete test and inspection progra subject to DSA requirements for special inspection or str to, special inspections not listed on this form such as str	noted on this form are those that will be perform am must be performed as detailed on the DSA a fuctural testing. The project inspector is response fuctural wood framing, high-load wood diaphragn Title 24, Part 2, Chapter 17A (20)	<u> </u>
**NOTE: Undefined section and table references	found in this document are from the CPC, or C	alifornia Puilding Codo

**NOTE: Undefined section and table references found in this document are from the CBC, or California Building Code.						
KEY TO COLUMNS						
1. TYPE	1. TYPE 2. PERFORMED BY					
inspection is required  Periodic – Indicates that a periodic special inspection is required  Test – Indicates that a test is required	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.  LOR – Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.  PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.  SI – Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.					

Performed By Code References and Notes

1905A.1.17; ACI 318-19 Section 26.12.

Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant

**1705.A.3.3.1**, or eliminated per **1705A.3.3.2**. See IR 17-13. (See Appendix

Sample and test anchor bolts and anchor rods not readily identifiable per

ocedures noted in DSA IR 17-11.

spection may be reduced to 'Periodic' subject to requirements in Section

		- 716 -		
<b>☑</b>	<ul> <li>a. Verify that:</li> <li>Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations.</li> <li>Foundation excavations are extended to proper depth and have reached proper material.</li> <li>Materials below footings are adequate to achieve the design bearing capacity.</li> </ul>	See Notes		Refer to specific items identified in the Appendix listing exemptions for limitations. Placement of controlled fill exceeding 12" depth under foundations is not permitted without a geotechnical report.
	S2. SOIL COMPACTION AND FILL:			
	Test or Special Inspection	Туре	Performed By	Code References and Notes
	a. Verify use of proper materials, densities and inspect lift	Continuous	LOR*	* Under the supervision of a geotechnical engineer or LOR's engineering
<b>✓</b>	thicknesses, placement and compaction during placement of fill.			manager. Refer to specific items identified in the Appendix listing exemptions for limitations.

	Test or Special Inspection	Туре	Performed By	Code References and Notes
<b>V</b>	Inspect drilling operations and maintain complete and accurate records for each pier.	Continuous	PI	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
<b>V</b>	<b>b.</b> Verify pier locations, diameters, plumbness, and lengths. Record concrete or grout volumes.	Continuous	PI	Continuous inspection to be provided by project inspector. Refer to specific items identified in the Appendix listing exemptions for limitations.
Z	c. Concrete piers.	Provide tests ar	nd inspections per	r CONCRETE section below.
	C1. CAST-IN-PLACE CONCRETE			Į.
	Test or Special Inspection	Туре	Performed By	Code References and Notes
<b>√</b>	a. Verify use of required design mix.	Periodic	SI	Table 1705A.3 Item 5, 1910A.1.
<b>V</b>	<b>b.</b> Identifiy, sample, and test reinforcing steel.	Test	LOR	1910A.2; ACI 318-19 Ch. 20 and Section 26.6.1.2; DSA IR 17-10. (See Appendix (end of this form) for exemptions.)
<b>√</b>	c. During concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Test	LOR	Table 1705A.3 Item 6; ACI 318-19 Sections 26.5 & 26.12.

	Test or Special Inspection	Туре	Performed By	Code References and Notes
V	<ul> <li>a. Verify identification of all materials and:</li> <li>Mill certificates indicate material properties that comply with requirements.</li> <li>Material sizes, types and grades comply with requirements.</li> </ul>	Periodic	*	<b>Table 1705A.2.1 Item 3a–3c.</b> 2202A.1; AISI S100-20 Section A3.1 & AISI S240-20 Section A3 & A5, AISI S220-20 Sections A4 & A6. * By special inspector or qualified technician when performed off-site.
<b>√</b>	b. Test unidentified materials	Test	LOR	2202A.1.
<b>√</b>	c. Examine seam welds of HSS shapes	Periodic	SI	DSA IR 17-3.
V	d. Verify and document steel fabrication per DSA-approved construction documents.	Periodic	SI	Not applicable to cold-formed steel light-frame construction, except for trusses (1705A.2.4).

See Notes

•	Test or Special Inspection	Туре	Performed By	Code References and Notes
V	a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.	Periodic	SI	<b>Table 1705A.2.1 Items 1a &amp; 1b, 2202A.1;</b> AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.
V	b. Test high-strength bolts, nuts and washers.	Test	LOR	Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.
V	c. Bearing-type ("snug tight") connections.	Periodic	SI	<b>Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2;</b> AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.
V	d. Pretensioned and slip-critical connections.	*	SI	<b>Table 1705A.2.1 Items 2b &amp; 2c, 1705A.2.6, 2204A.2;</b> AISC 360- 16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.

Performed By Code References and Notes

	rear at a position inspection.	1.760				
V	<b>a.</b> Verify weld filler material identification markings per AWS designation listed on the DSA- approved documents and the WPS.	Periodic	SI	1705A.2.5, Table 1705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3.		
V	<b>b.</b> Verify weld filler material manufacturer's certificate of compliance.	Periodic	SI	DSA IR 17-3.		
<b>√</b>	c. Verify WPS, welder qualifications and equipment.	Periodic	SI	DSA IR 17-3.	Τ	
					T	
-	S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):					
	Test or Special Inspection	Туре	Performed By	Code References and Notes		
<b>V</b>	<ul> <li>a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds &gt; 5/16", plug and slot welds.</li> </ul>	Continuous	SI	<b>Table 1705A.2.1 Items 5a.1–4</b> ; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.		
<b>V</b>	<b>b.</b> Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	Periodic	SI	<b>1705A.2.2, Table 1705A.2.1 Items 5a.5 &amp; 5a.6</b> ; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.		
	S/A9. ANCHOR BOLTS AND ANCHOR RODS:	-			Τ	
	Test or Special Inspection	Туре	Performed By	Code References and Notes	1	

Name	of Architect or Engineer in general responsi	ble charge:

Name of Structural Engineer (When structural design has been delegated)

Signature of Architect or Structural Engineer:

a. Anchor Bolts and Anchor Rods

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures

DSA STAMP

#### DSA 103-22: LIST OF REQUIRED VERIFIED REPORTS, CBC 2022

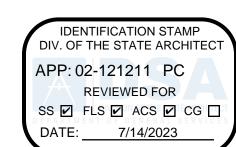
- . Soils Testing and Inspection: Geotechnical Verified Report Form DSA 293 Structural Testing and Inspection: Laboratory Verified Report Form DSA 29
- 3. Shop Welding Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292 4. High-Strength Bolt Installation Inspection: Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292

STATE APPROVALS-SITE



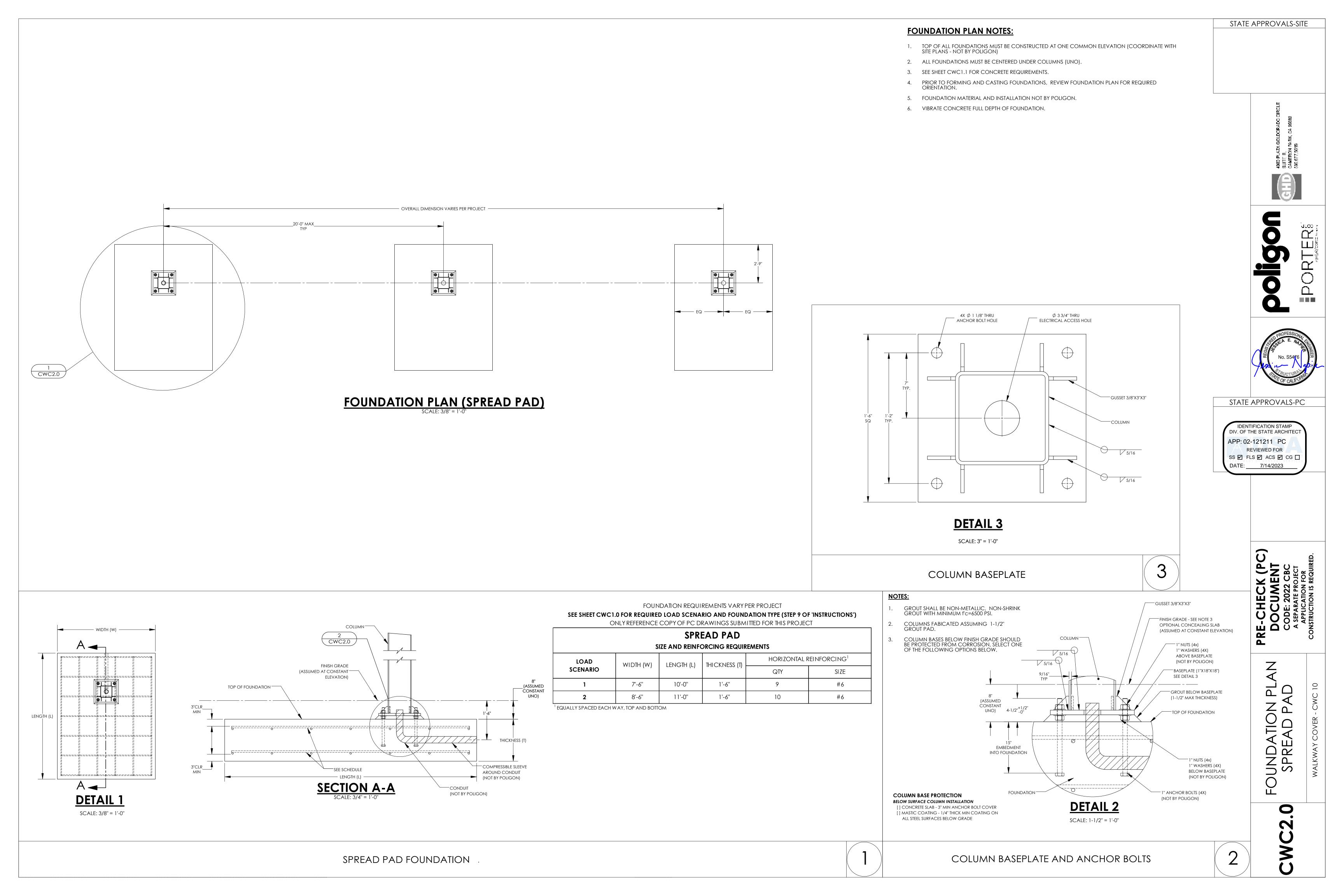




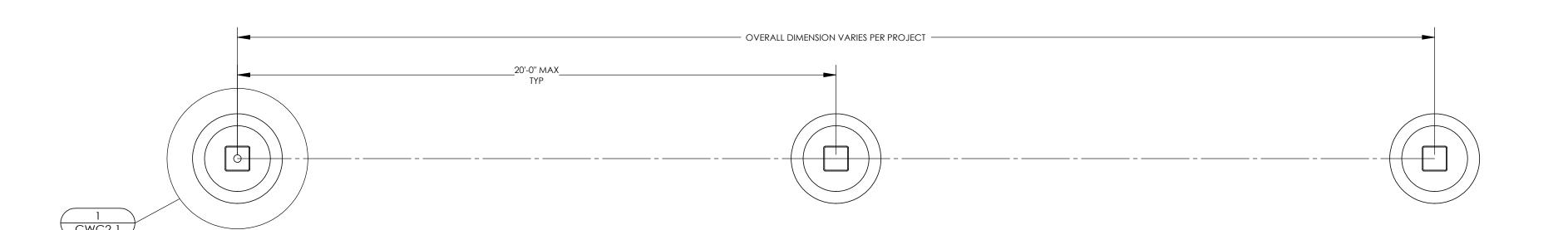


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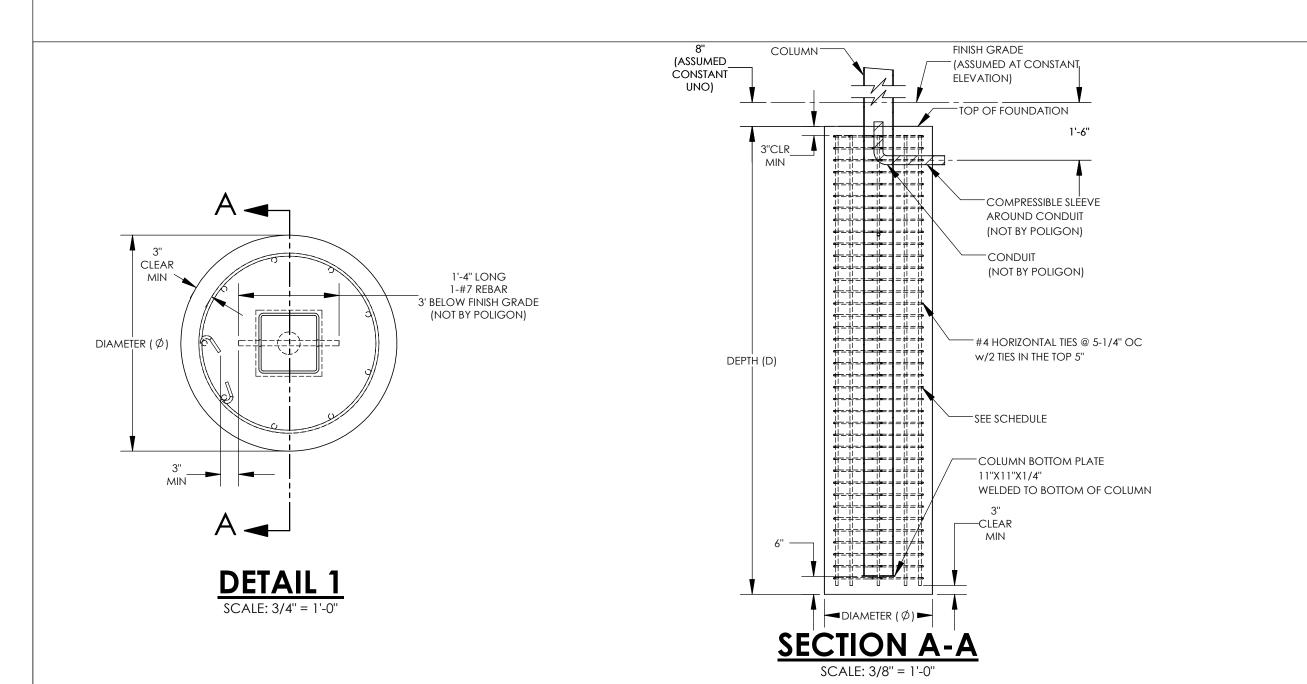
 $\Box$ 



- TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLIGON)
- 2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
- 3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
- 4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED
- 5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLIGON.
- 6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
- 7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).



## FOUNDATION PLAN (DRILLED PIER) SCALE: 3/8" = 1'-0"



FOUNDATION REQUIREMENTS VARY PER PROJECT

SEE SHEET CWC1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')

ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

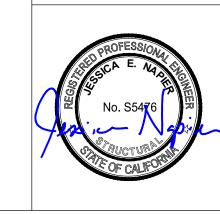
SIZE AND REINFORCING REQUIREMENTS						
LOAD	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING <sup>1</sup>			
SCENARIO			QTY	SIZE		
1	3'-0''	13'-6"	9	#7		
2	3'-0"	15'-0''	9	#7		

STATE APPROVALS-SITE

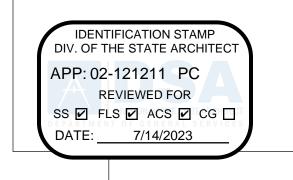
H B SUITE B.
CAMERON PARK, CA 16682







STATE APPROVALS-PC



PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC

OUNDATION PLAN PRILLED PIER

**SWC2.1** 

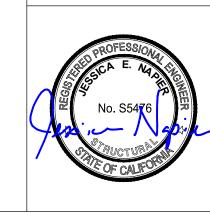


- TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS NOT BY POLIGON)
- 2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
- 3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
- PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED ORIENTATION.
- FOUNDATION MATERIAL AND INSTALLATION NOT BY POLIGON.
- VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.

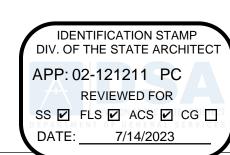


STATE APPROVALS-SITE









PRE-CHE DOCU CODE: 2 A SEPARAT APPLICA CONSTRUCTIO

SPREAD PA

2

6X Ø 1 1/8" THRU ANCHOR BOLT HOLE Ø 3 3/4" THRU ELECTRICAL ACCESS HOLE GUSSET 3/8" X 3" TALL X 3-1/2" LONG **DETAIL 3** SCALE: 1/4" = 1'-0" COLUMN BASEPLATE

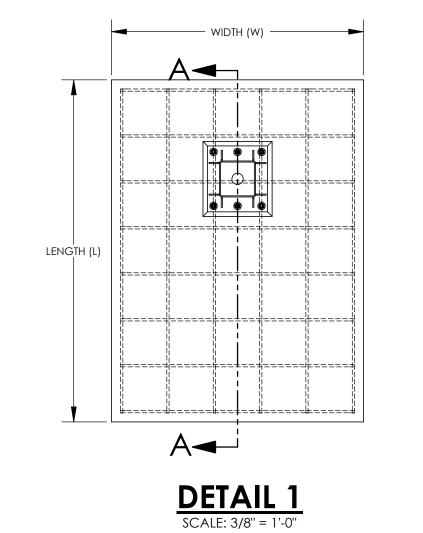
— OVERALL DIMENSION VARIES PER PROJECT — 16'-0" MAX FOUNDATION PLAN (SPREAD PAD)
SCALE: 3/8" = 1'-0"

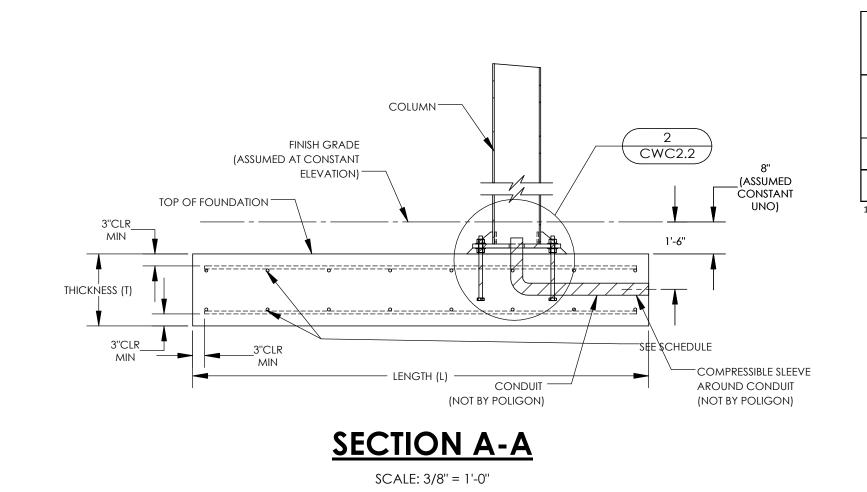
FOUNDATION REQUIREMENTS VARY PER PROJECT

SEE SHEET CWC 1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS') ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

SPREAD PAD SIZE AND REINFORCING REQUIREMENTS						
LOAD	WIDTH (W)	LENGTH (L)	THICKNESS (T)	HORIZONTAL REINFORCING <sup>1</sup>		
SCENARIO				QTY	SIZE	
1	8'-6"	10'-0''	1'-6''	9	#6	
2	9'-6"	11'-6"	1'-6"	11	#6	
1						

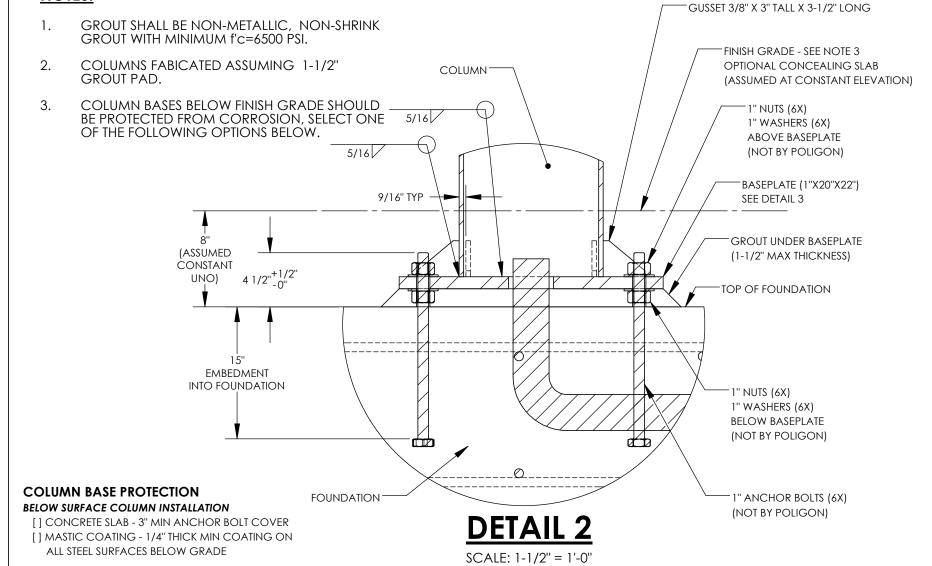
<sup>1</sup> EQUALLY SPACED EACH WAY, TOP AND BOTTOM

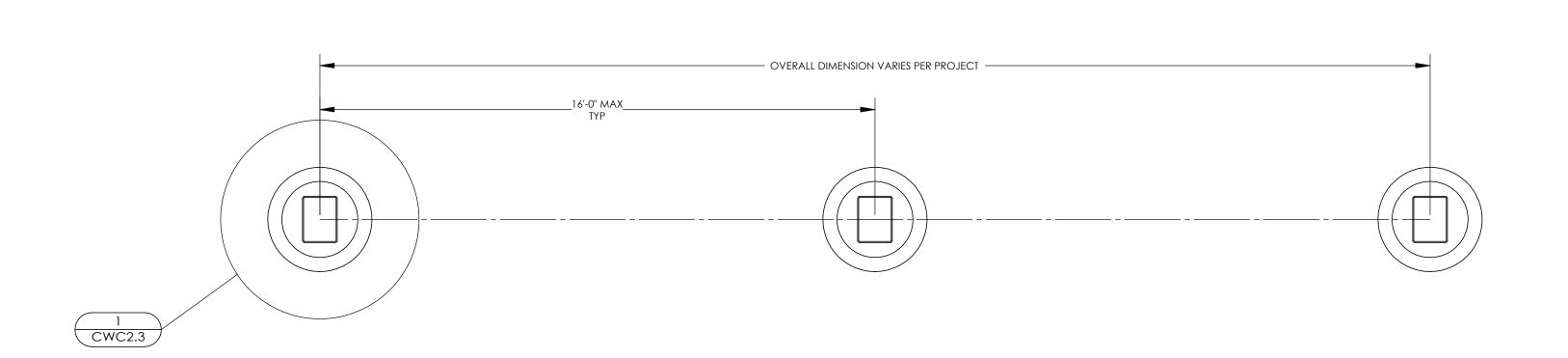




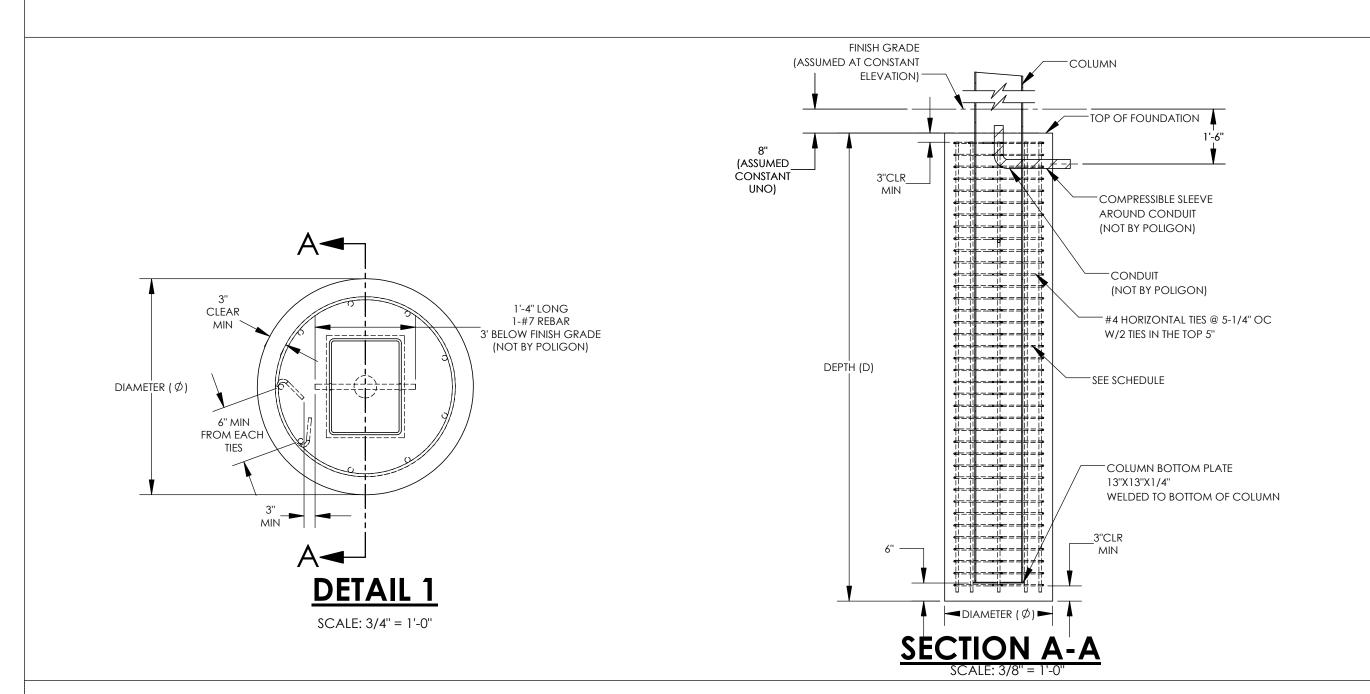


NOTES:





## FOUNDATION PLAN (DRILLED PIER) SCALE: 3/8" = 1'-0"



FOUNDATION REQUIREMENTS VARY PER PROJECT

SEE SHEET CWC 1.0 FOR REQUIRED LOAD SCENARIO AND FOUNDATION TYPE (STEP 9 OF 'INSTRUCTIONS')

ONLY REFERENCE COPY OF PC DRAWINGS SUBMITTED FOR THIS PROJECT

DRILLED PIER							
SIZE AND REINFORCING REQUIREMENTS							
LOAD	DIAMETER (Ø)	DEPTH (D)	VERTICAL REINFORCING <sup>1</sup>				
SCENARIO	DIAMEILK (Ø)	DEI III (D)	QTY	SIZE			
1	3'-0"	15'-0''	9	#7			
2	3'-0"	17'-0''	9	#7			
1 EQUALLY SPACED AROUND DRILLED PIER							

DRILLED PIER FOUNDATION

#### **FOUNDATION PLAN NOTES:**

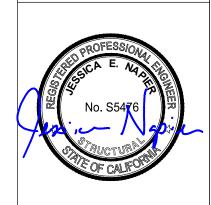
- TOP OF ALL FOUNDATIONS MUST BE CONSTRUCTED AT ONE COMMON ELEVATION (COORDINATE WITH SITE PLANS - NOT BY POLIGON)
- 2. ALL FOUNDATIONS MUST BE CENTERED UNDER COLUMNS (UNO).
- 3. SEE SHEET CWC1.1 FOR CONCRETE REQUIREMENTS.
- 4. PRIOR TO FORMING AND CASTING FOUNDATIONS, REVIEW FOUNDATION PLAN FOR REQUIRED
- 5. FOUNDATION MATERIAL AND INSTALLATION NOT BY POLIGON.
- 6. VIBRATE CONCRETE FULL DEPTH OF FOUNDATION.
- 7. FOR DRILLED PIER FOUNDATIONS, PREVENT SOIL FROM ENTERING EXCAVATED HOLE (FORM, ETC).

4080 PLAZA GOLDORADO CIRCI SUITE B. CAMERON PARK, CA 96882

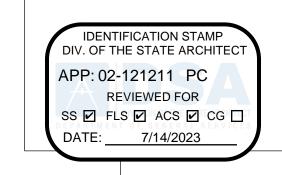
STATE APPROVALS-SITE







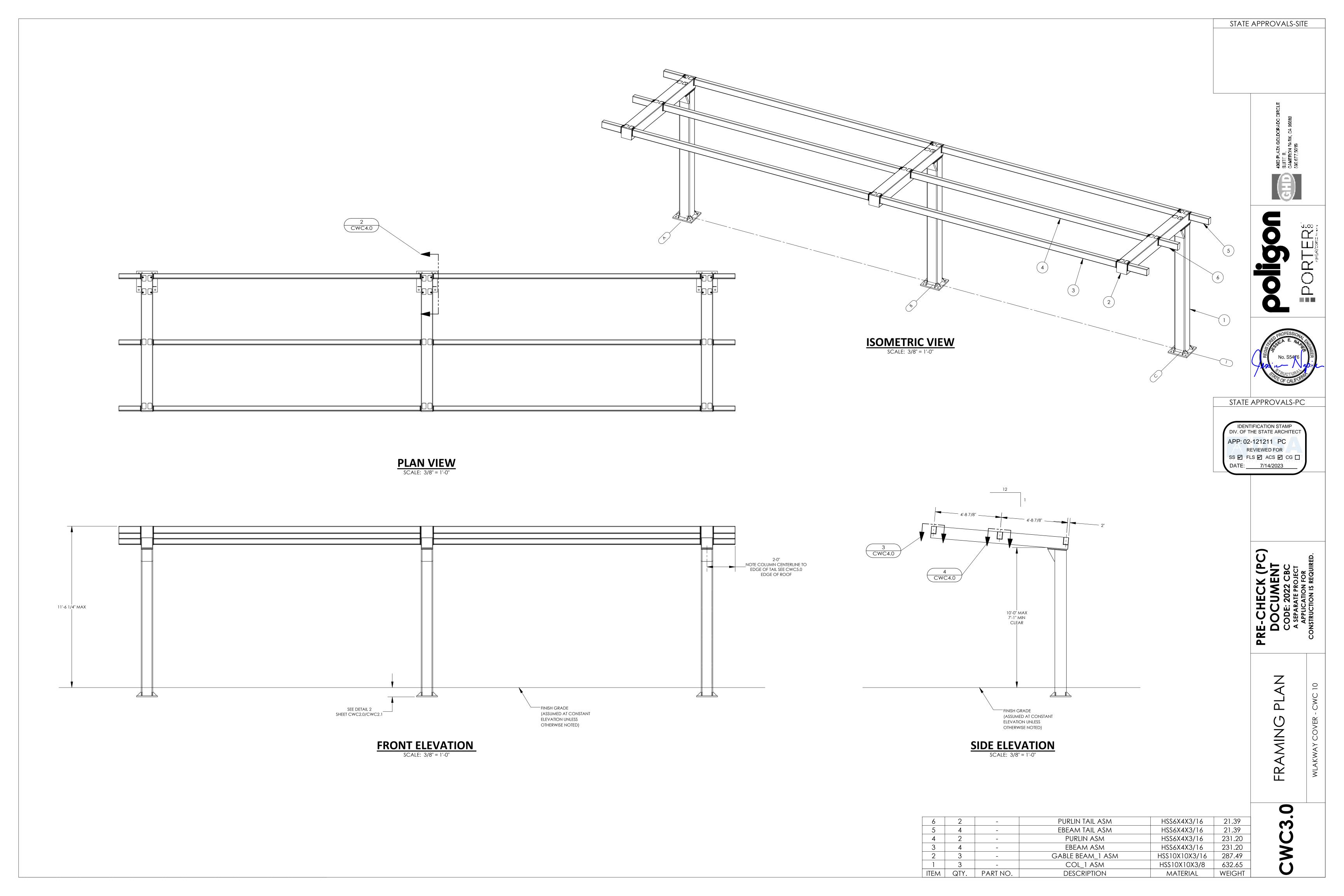
STATE APPROVALS-PC

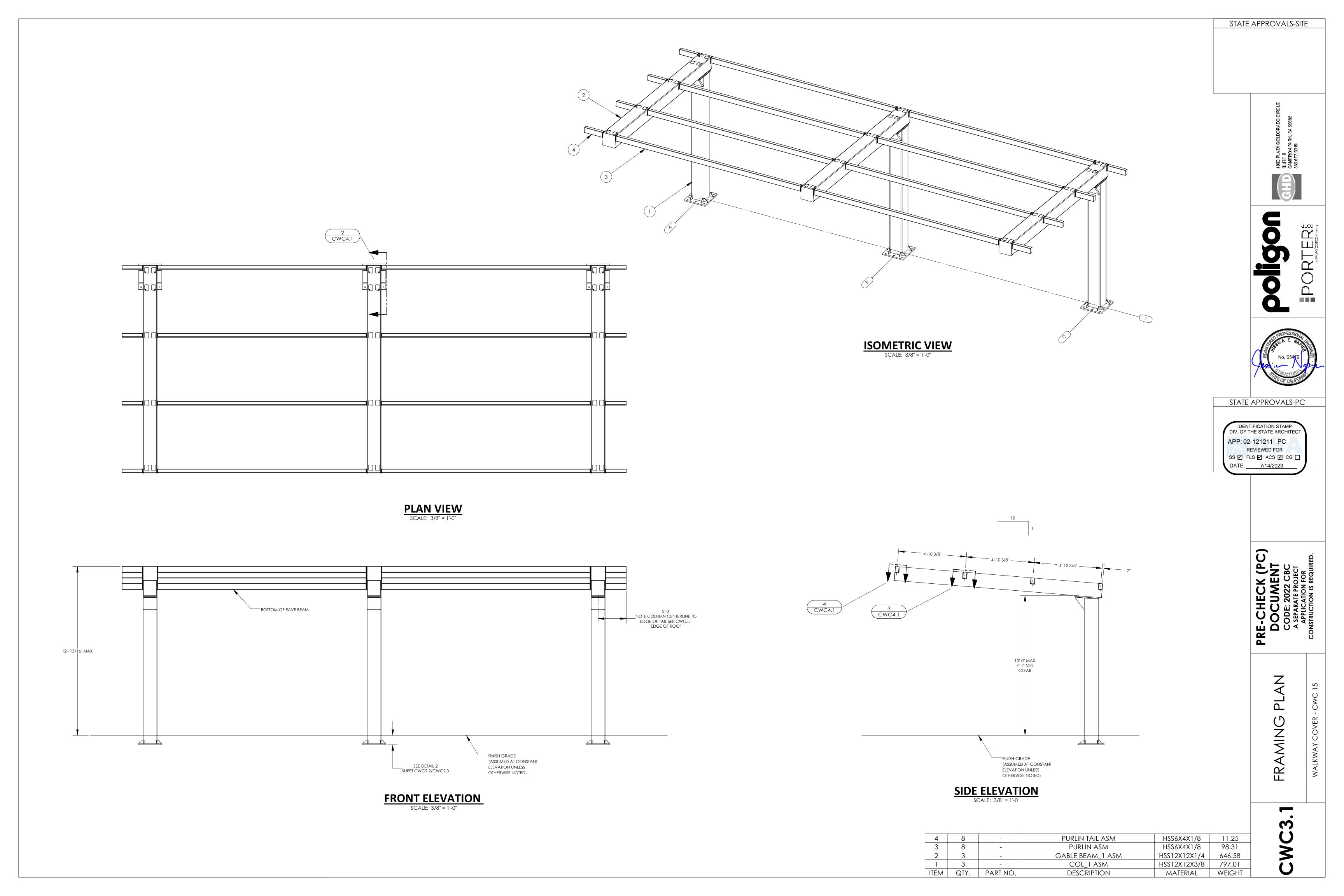


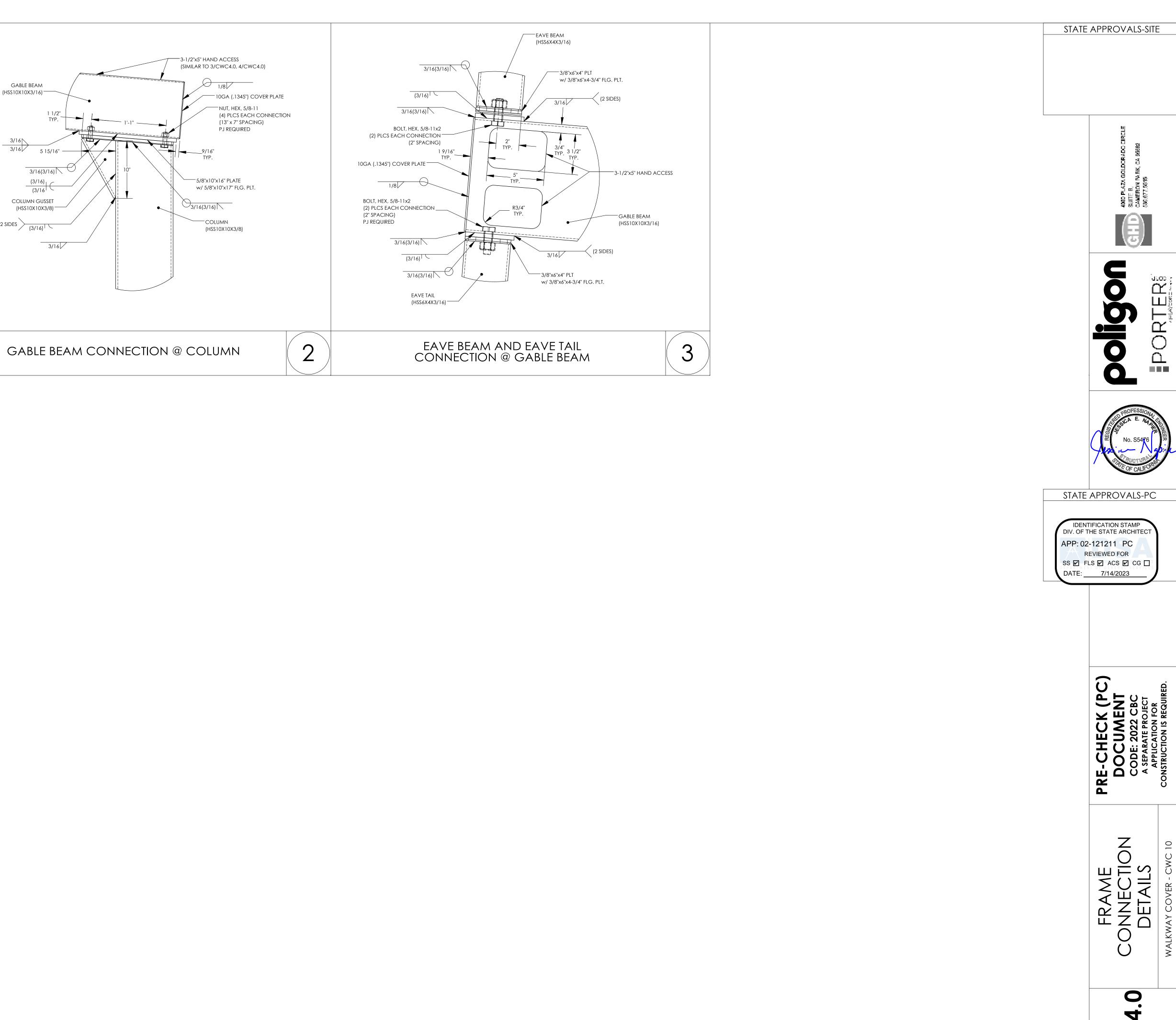
PRE-CHECK (PC)
DOCUMENT
CODE: 2022 CBC
A SEPARATE PROJECT

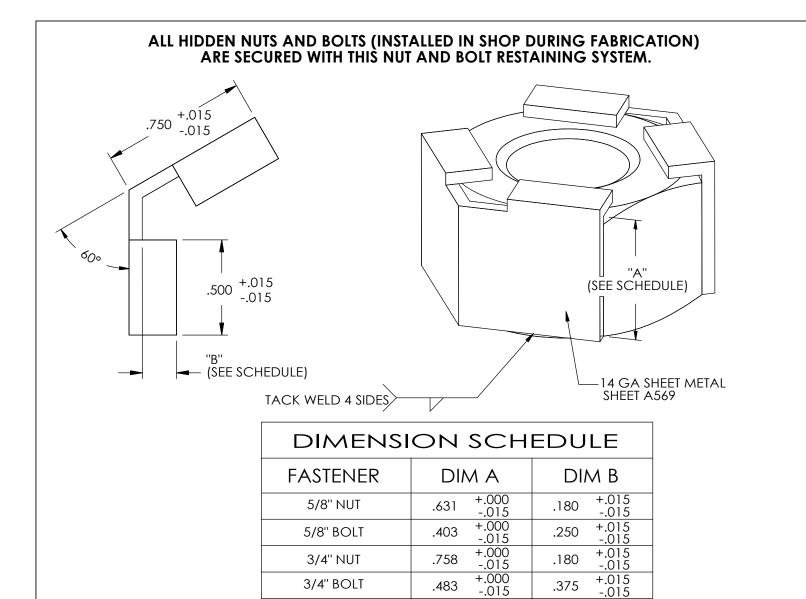
FOUNDATION PLAN DRILLED PIER

**CWC2.3** 









GABLE BEAM (HSS10X10X3/16)-

5 15/16" —

3/16(3/16)

---COLUMN

3/16(3/16)

(3/16)

COLUMN GUSSET

2 SIDES (3/16)

(HSS10X10X3/8) —

# NUT & BOLT RESTRAINING SYSTEM

1" NUT

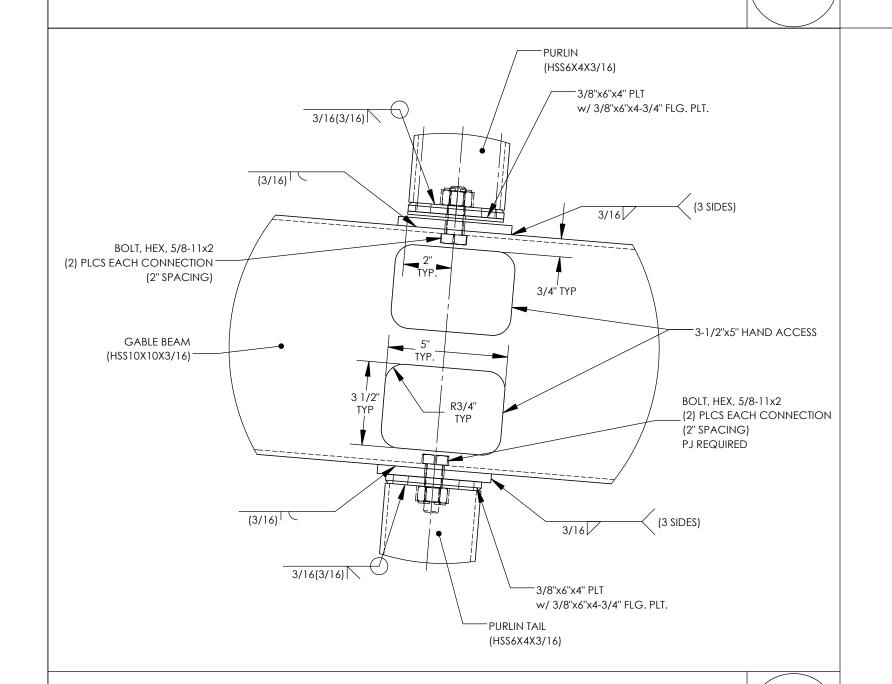
1" BOLT

1.012 +.000

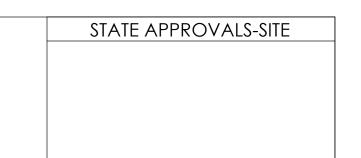
.643 +.000 -.015

.180

.375 +.015

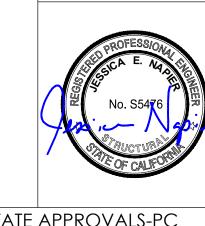


PURLIN AND PURLIN TAIL CONNECTION @ GABLE BEAM

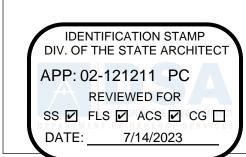








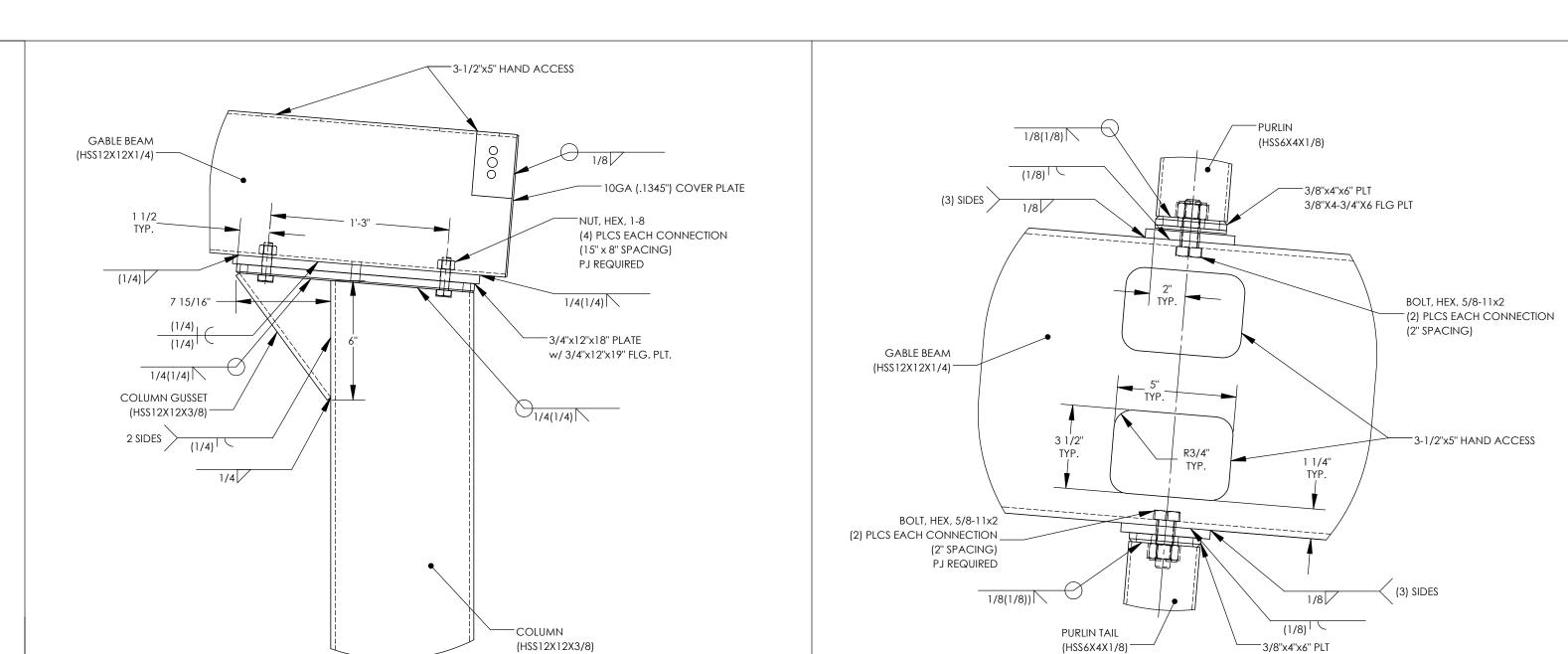
STATE APPROVALS-PC



FRAME ONNECTION DETAILS

4

CWC



2

GABLE BEAM CONNECTION @ COLUMN

3/8"X4-3/4"X6 FLG PLT

PURLIN AND PURLIN TAIL

CONNECTION @ GABLE BEAM



TACK WELD 4 SIDES

**FASTENER** 

5/8" NUT

5/8" BOLT

3/4" NUT

3/4" BOLT

1" NUT

1" BOLT

.500 +.015

✓ (SEE SCHEDULE)

ALL HIDDEN NUTS AND BOLTS (INSTALLED IN SHOP DURING FABRICATION)
ARE SECURED WITH THIS NUT AND BOLT RESTAINING SYSTEM.

DIMENSION SCHEDULE

DIM A

.631 +.000

.403 +.000

1.012 +.000

.643 +.000

.758

.483

(SEE SCHEDULE)

DIM B

.180

.250

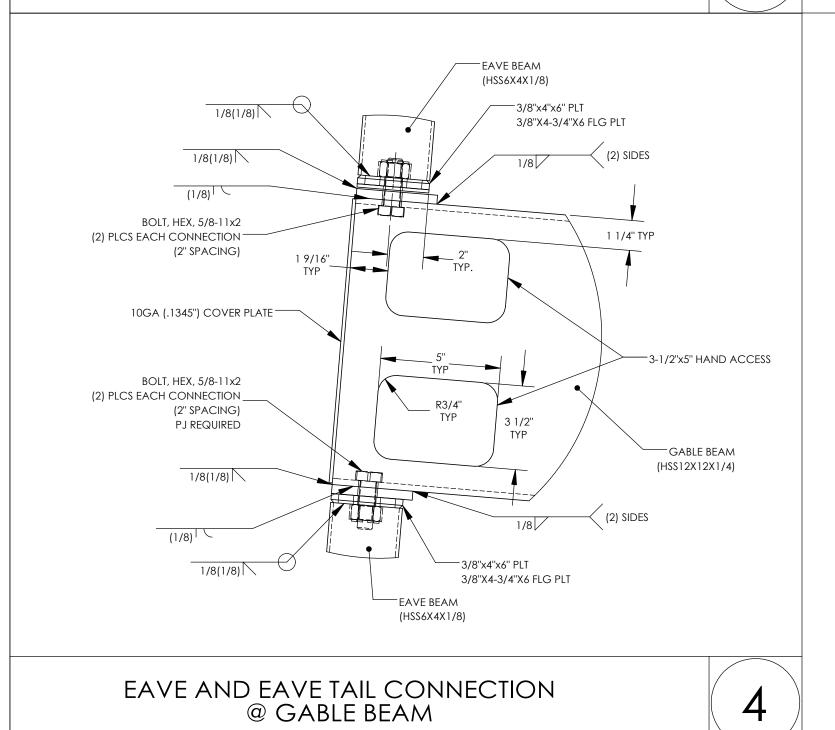
.180

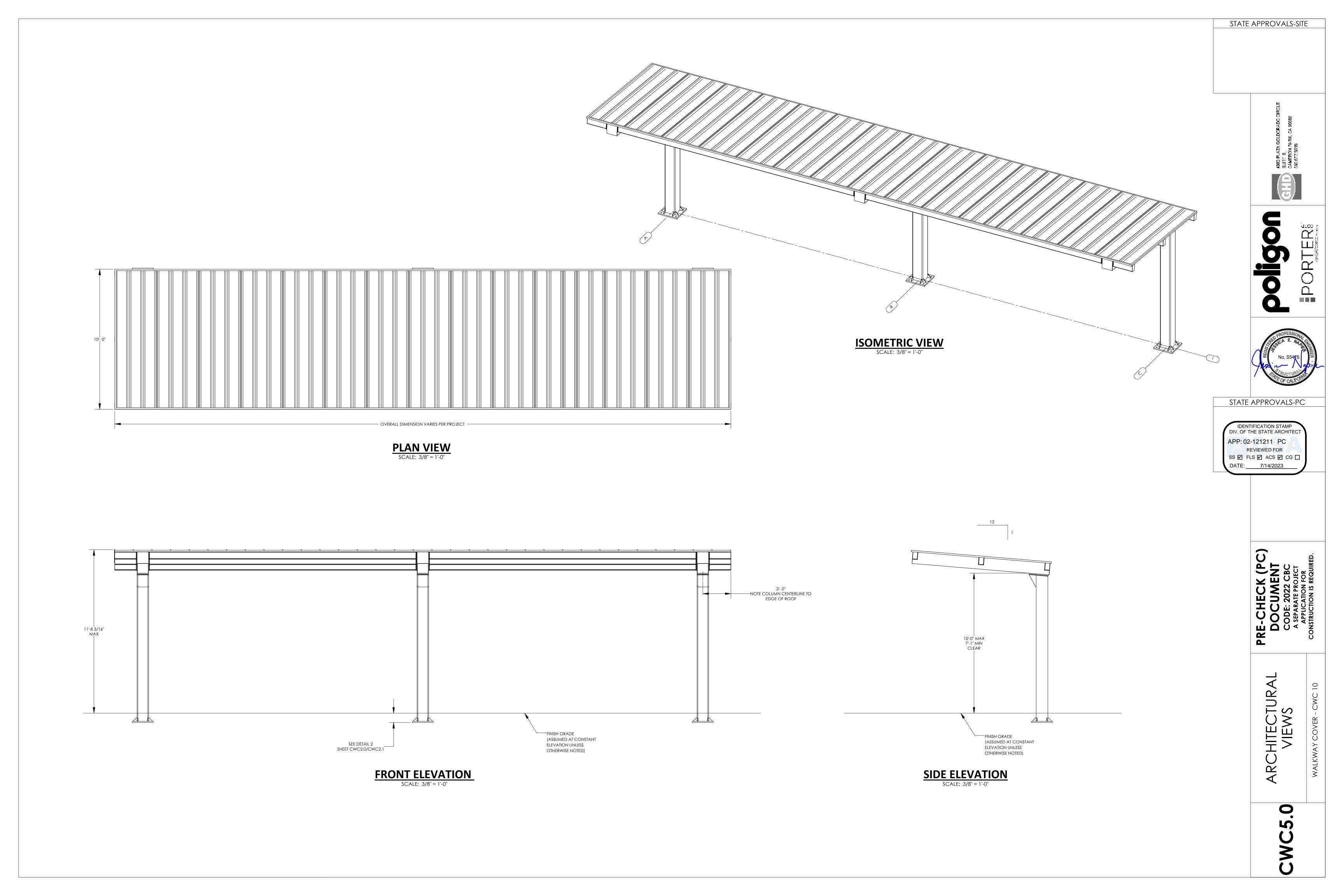
.375

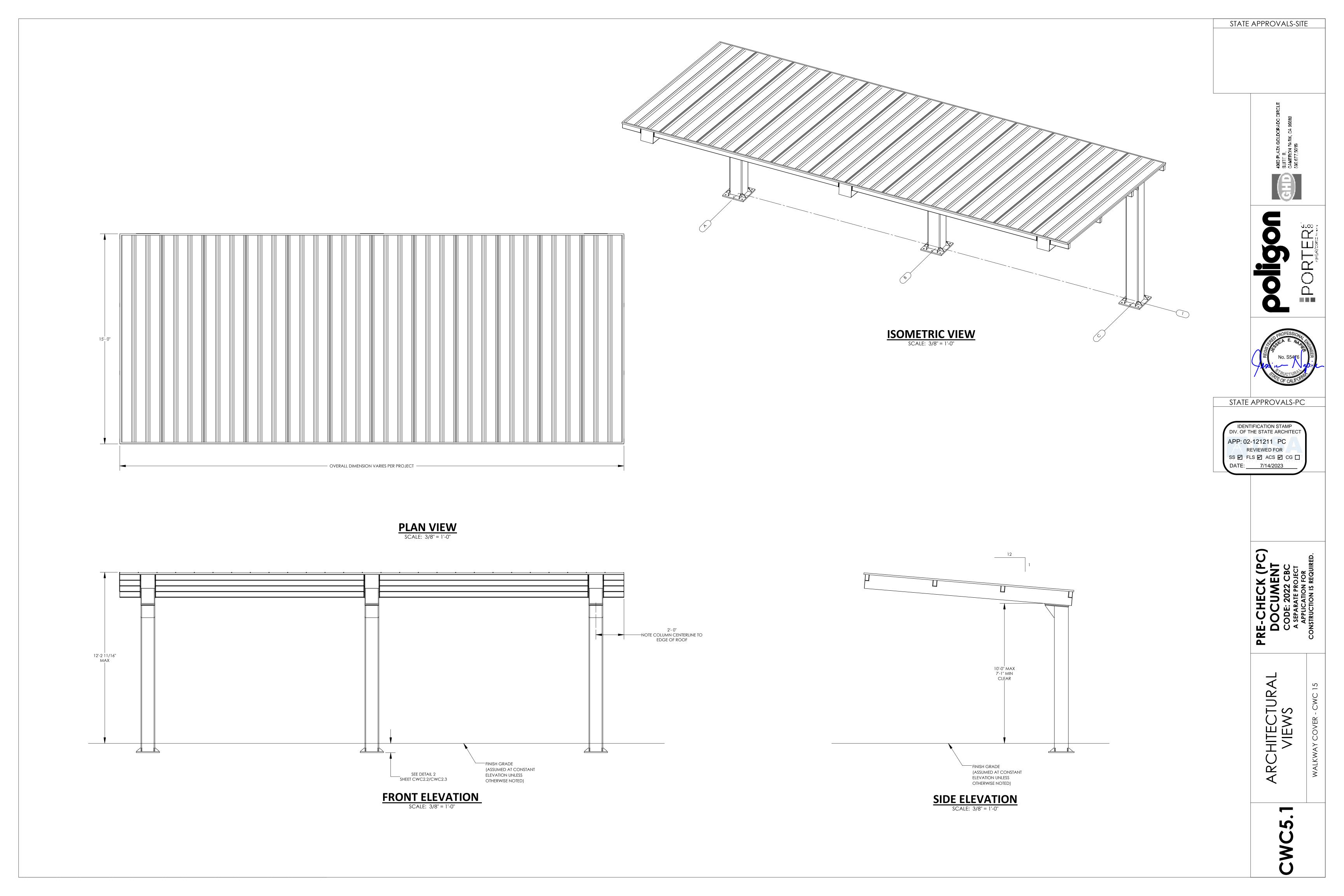
.180

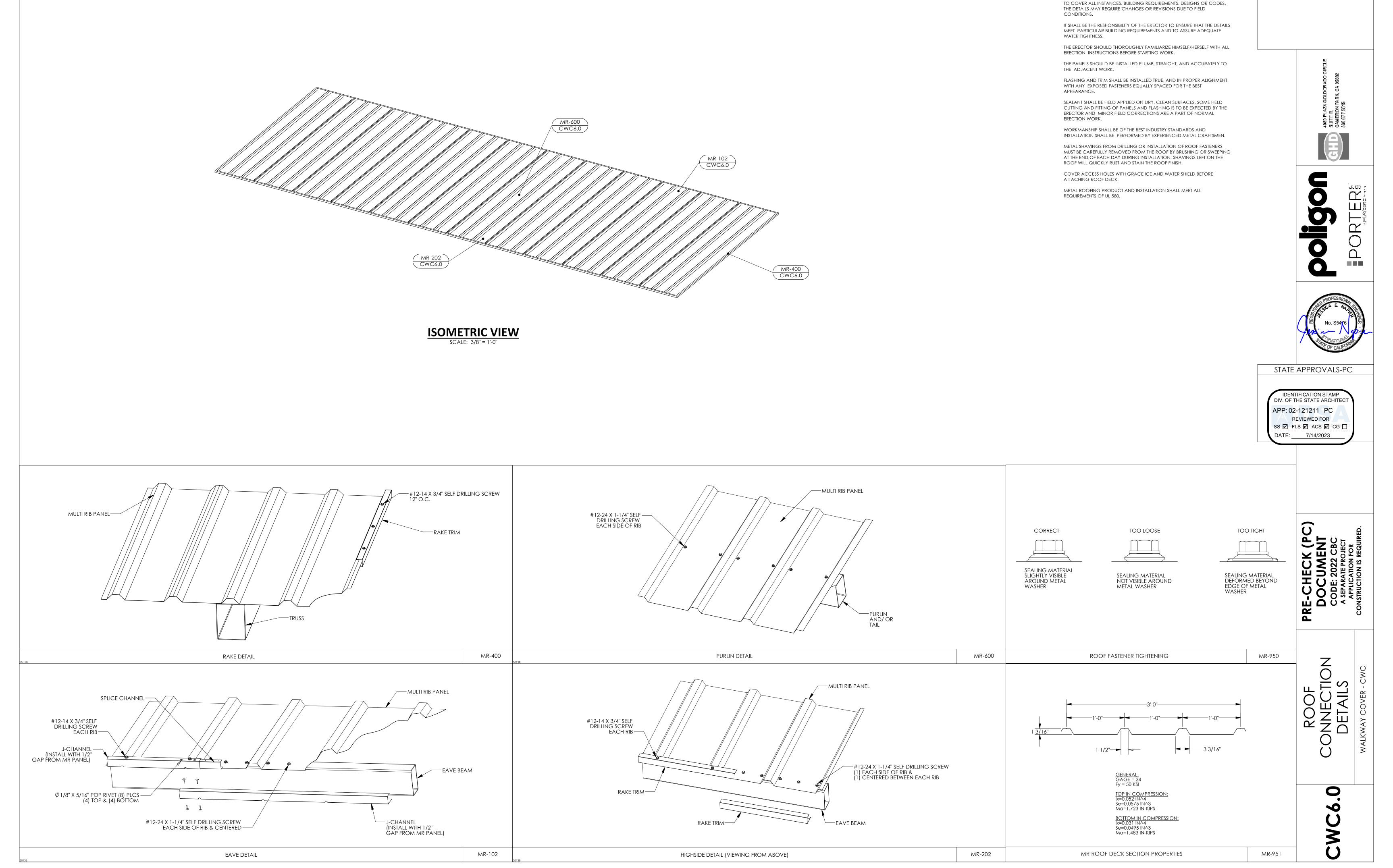
.375 +.015

—14 GA SHEET METAL SHEET A569





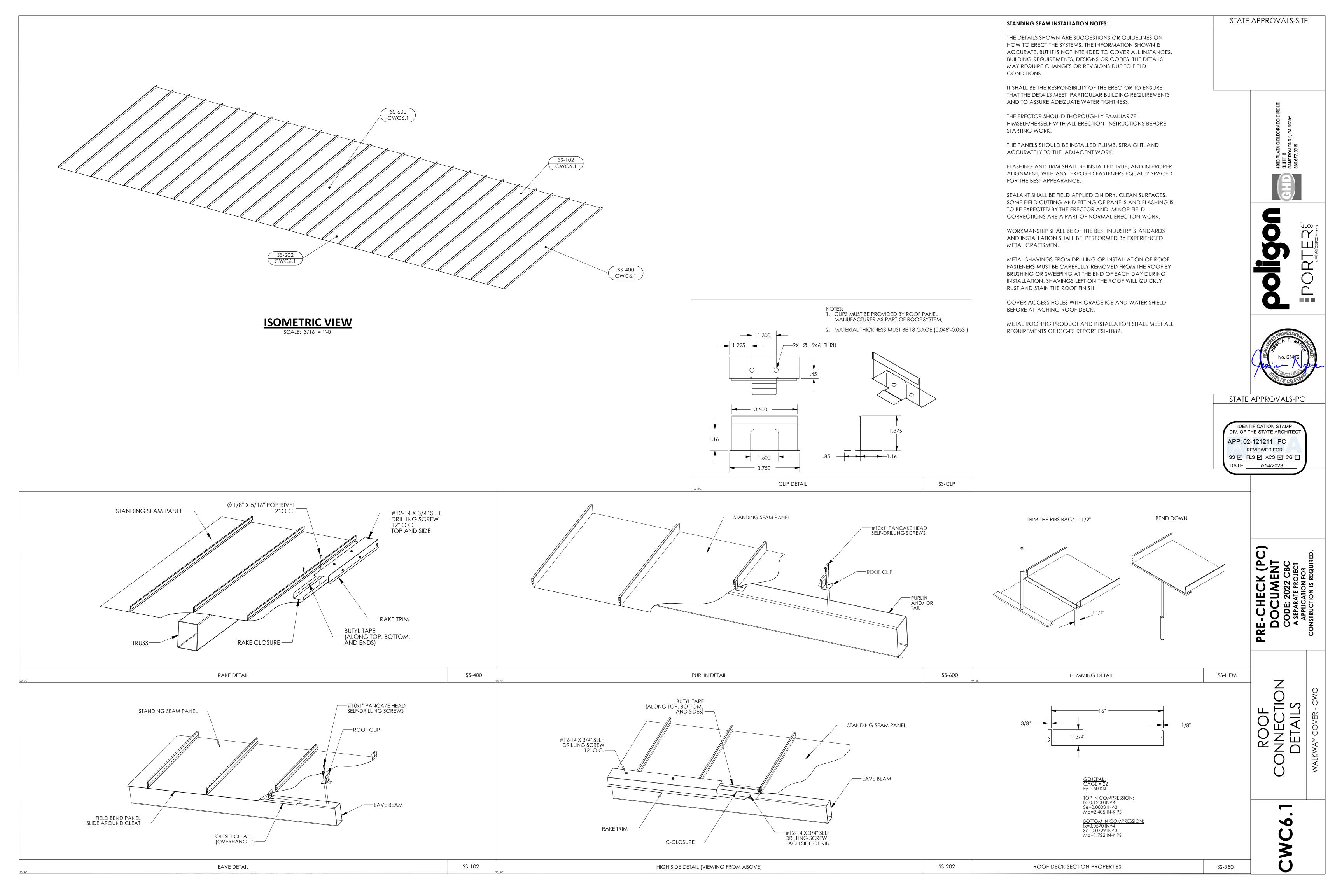




STATE APPROVALS-SITE

MULTI-RIB NOTES:

THE DETAILS SHOWN ARE SUGGESTIONS OR GUIDELINES ON HOW TO ERECT THE SYSTEMS. THE INFORMATION SHOWN IS ACCURATE, BUT IT IS NOT INTENDED



# ROOF PANEL-POLIGUARD GUTTER (SLOPE PER INSTALLATION INSTRUCTIONS) POLIGUARD GUTTER HANGER – 4" FROM ENDS @ 24" OC MAX **GUTTER DETAIL** GS-100 -ROOF PANEL —EAVE BEAM (VARIES) POLIGUARD GUTTER (SLOPE PER INSTALLATION INSTRUCTIONS) POLIGUARD DOWNSPOUT COLUMN — (VARIES) -POLIGUARD DOWNSPOUT BRACKET 6" FROM ELBOW 7'-1" MIN CLEAR TO BOTTOM OF DOWNSPOUT FINISH GRADE -DOWNSPOUT DETAIL GS-200 5 3/4" EGDE OF ROOF TO FACE OF GUTTER ATTACH (2) #10X1" PANCAKE HEAD SELF-DRILLING SCREWS GUTTER **GUTTER DETAIL** GS-300

### **POLIGUARD GUTTER SYSTEM NOTES:**

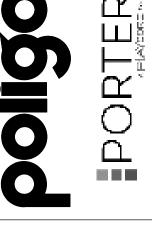
- 1. PREFABRICATED GUTTER SYSTEM IS ATTACHED TO THE STRUCTURE AFTER ROOF IS INSTALLED.
- 2. DETAILED INSTALLATION INSTRUCTIONS ARE SHIPPED WITH THE STRUCTURE.
- 3. DOWNSPOUTS REQUIRED AT EACH COLUMN.

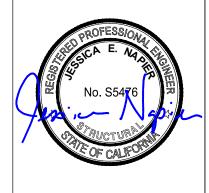
#### **ELECTRICAL CUTOUT NOTES:**

- 1. MAXIMUM ONE CUTOUT PERMITTED IN EACH MEMBER.
- 2. CUTOUTS CAN BE PLACED ON ANY SIDE OF A MEMBER.
- 3. CUTOUTS CAN BE PLACED ALONG MEMBERS AS INDICATED IN THE DETAILS.
- 4. ARCHITECTS REQUESTING CUTOUTS MUST MARKUP APPROVED PC DRAWINGS TO LOCATE CUTOUTS FOR APPROVAL AND FABRICATION.

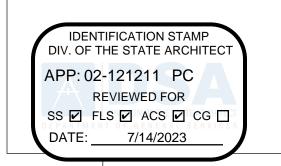
STATE APPROVALS-SITE







STATE APPROVALS-PC



MISC DESIGN OPTIONS



2 1/4"----

SEE DETAIL 2 SHEET CWC2.0 & CWC2.2

(ASSUMED AT CONSTANT ELEVATION UNLESS

EC-100

OTHERWISE NOTED)

CWC7.1

ELECTRICAL CUTOUT AND ACCESS INSTRUCTIONS

- IF 'YES' IS NOT SELECTED IN STEP 2 ON ORDER FORM, THEN THIS SHEET NEED

NOT BE INCLUDED IN SITE-SPECIFIC DRAWINGS

- ONLY COLUMNS ARE PERMITTED TO HAVE ELECTRICAL ACCESS

- THE COLUMN CUTOUTS ARE STATIC AND SHOWN IN THE 'MISC DESIGN OPTIONS SHEET'

- IDENTIFY THE COLUMNS WITH ELECTRICAL CUTOUTS BELOW (REFERENCE GRID LINES IN

ISOMETRIC FRAME VIEW TO THE RIGHT)

- STRUCTURES MAY BE LONGER OR SHORTER THAN THE ISOMETRIC FRAME VIEW SHOWN

- IF SITE-SPECIFIC STRUCTURE HAS A DIFFERENT NUMBER OF COLUMNS THAN ISOMETRIC SHOWN,

REFERENCE COLUMN A1 IN THE ISOMETRIC VIEW AND CONTINUE PATTERN TO FIT SITE-SPECIFIC LAYOUT

- IF NO COLUMNS ARE IDENTIFIED, POLIGON WILL ASSUME CUTOUTS ONLY IN COLUMN A1

- CONTACT POLIGON ENGINEERING FOR SPECIAL PROJECT SPECIFIC REQUIREMENTS

# SPECIFIC MEMBERS

### **EXAMPLE**:

## **ELECTRICAL CUTOUT IDENTIFICATION IN COLUMNS**

SPECIFIC MEMBERS

A1, B1, F1

